



1 **Cohesive and mixed sediment in the Regional Ocean Modeling**
2 **System (ROMS v3.6) implemented in the Coupled Ocean**
3 **Atmosphere Wave Sediment-Transport Modeling System**
4 **(COAWST r1179)**

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15 **Abstract.** We describe and demonstrate algorithms for treating cohesive and mixed sediment that have been added to the
16 Regional Ocean Modeling System (ROMS version 3.6), as implemented in the Coupled Ocean Atmosphere Wave Sediment-
17 Transport Modeling System (COAWST Subversion repository revision 1179). These include: flocculation dynamics (aggregation
18 and disaggregation in the water column); changes in flocculation characteristics in the seabed; erosion and deposition of cohesive
19 and mixed (combination of cohesive and non-cohesive) sediment; and bioturbative mixing of bed sediment. These routines
20 supplement existing non-cohesive sediment modules, thereby increasing our ability to model fine-grained and mixed-
21 sediment environments. Additionally, we describe changes to the sediment bed-layering scheme that improve the fidelity of
22 the modeled stratigraphic record. Finally, we provide examples of these modules implemented in idealized test cases and a
23 realistic application.

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27 **1 Introduction**

28 **1.1 Motivation**

29 Fine cohesive sediment (mud) is present in almost every coastal environment, and influences water clarity, benthic habitats,
30 shoaling of harbors and channels, storage and transport of nutrients and contaminants, and morphologic evolution of
31 wetlands, deltas, estuaries, and muddy continental shelves (Winterwerp and van Kesteren, 2004; Edmonds and Slingerland,
32 2010; Caldwell and Edmonds, 2014; Mehta, 2014; Li et al., 2017). The properties and behavior of mud depend on more than
33 the size, shape, and density of the individual particles, so they are more difficult to characterize and model than properties of
34 non-cohesive material like sand. Cohesive sediment often forms flocs that have lower densities, larger diameters, and faster
35 settling velocities than the primary particles. Acoustic and optical sensors respond differently to suspensions of flocculated
36 sediment, compared with similar mass concentrations of unflocculated particles, and these responses have important
37 influences on observations of suspended-sediment mass concentrations, especially in estuaries (for example, McCave and
38 Swift, 1976; McCave, 1984; Eisma, 1986; Hill and Nowell, 1995; Winterwerp, 1999, 2002; Winterwerp et al., 2006; Xu,
39 Wang, and Riemer, 2008; 2010; Verney et al., 2011; Slade, Boss, and Russo, 2011; MacDonald et al., 2013; Thorne et al.,
40 2014).

41 Cohesive sediment beds are distinguished by generally finer sediment, including some clay content, often are poorly sorted,
42 and have low bulk density (high water content). Cohesive beds have a tendency for bulk responses to bottom stress, rather
43 than individual particle responses. Cohesive beds have rheological properties that can range from fluids to Bingham plastics
44 to granular materials, and may change with time in response to changes in water content, biochemical processes and fluid or
45 geomechanical stresses (Dyer, 1986; Whitehouse et al., 2000; Winterwerp and Kranenburg, 2002; Winterwerp and van
46 Kesteren, 2004; Maa et al., 2007; Knoch and Malcherek, 2011; Mehta, 2014).

47 Sediment transport in coastal ocean models is sensitive to the representation of fine-scale stratigraphy because evolving
48 seabed properties determine what sediment is exposed to the water column and available for transport. Small-scale
49 stratigraphy and grain-size distribution at the sediment-water interface also influence the grain roughness of the seabed,
50 affect the type of small-scale roughness (biogenic features and ripples) present on the bed, and control properties like
51 acoustic impedance of the seafloor. Biodiffusion influences stratigraphy by reducing gradients in grain size and other bed
52 properties and by mixing materials from deeper in the bed to closer to the surface, where they may be more susceptible to
53 transport.



54 1.2 Previous Modeling Efforts

55 Amoudry and Souza (2011) surveyed regional-scale sediment-transport and morphology models, and found that one of the
56 shortcomings was the treatment of cohesive- and mixed-sediment models. The water-column behavior of cohesive sediment
57 (e.g., flocculation and disaggregation, and settling) and the consolidation of settling particles to form a cohesive bed has been
58 modeled mostly with one-dimensional vertical (1DV) models or with empirical formulae that allow particle settling velocity
59 to vary as a function of salinity (Ralston et al., 2012) or suspended-sediment concentration (e.g., Mehta, 1986; Lick et al.,
60 1993; Van Leussen, 1994; Lumborg and Windelin, 2003; Lumborg, 2005; and Lumborg and Pejrup, 2005). The primary
61 dynamical effect of flocculation is to increase settling velocities, thereby increasing the mass settling flux. Soulsby et al.
62 (2013) reviewed methods for estimating floc settling velocities and proposed a new formulation that depends primarily on
63 turbulence shear and instantaneous suspended-sediment concentration. Spearman et al. (2011) noted that adjustments to
64 settling velocity (e.g., Manning and Dyer, 2007) were able to successfully reproduce floc settling in one-dimensional estuary
65 modeling applications. However, these approaches do not allow analysis of other characteristics of the suspended particle
66 field, such as acoustic and optical properties or geochemical properties (water content and surface area). Full floc dynamics
67 have been incorporated in only a few coastal hydrodynamics and sediment-transport models. Winterwerp (2002)
68 incorporated his floc model (Winterwerp, 1999) in a three-dimensional simulation of the estuary turbidity maximum (ETM)
69 in the Ems estuary. Ditschke and Markofsky (2008) described formulations in TELEMAC-3D to represent exchanges among
70 size classes from floc dynamics. Xu et al. (2010) added floc dynamics to the Princeton Ocean Model (POM) and simulated
71 the ETM in Chesapeake Bay.

72 Empirical formulae for the erosion of cohesive sediment have been derived from laboratory flume measurements and field
73 experiments (Whitehouse et al, 2000; Mehta, 2014). Many have a form similar to the Ariathurai and Arulanandan (1978)
74 equation used in ROMS (Warner et. al., 2008), which relates erosional flux E ($\text{kg m}^{-2} \text{s}^{-1}$) to the normalized excess shear
75 stress as $E = E_0(1 - \phi) [(\tau_{sf} - \tau_c) / \tau_{sf}]$ when $\tau_{sf} > \tau_c$, and where E_0 ($\text{kg m}^{-2} \text{s}^{-1}$) is an empirical rate constant, ϕ (m^3/m^3) is
76 sediment porosity, τ_{sf} (Pa) is the skin-friction component of the bottom shear stress, and τ_c (Pa) is the critical shear stress
77 for erosion. Erosion of cohesive sediment in some models (for example Delft3D; van der Wegen et al., 2011; Caldwell and
78 Edmonds, 2014) uses a similar formulation, subject to a user-specified critical shear stress for erosion. It is recognized that
79 that τ_c may increase with depth in sediment, and erosion-rate formulae have been proposed that incorporate depth-
80 dependent profiles for E_0 and/or τ_c (Whitehouse et al, 2000; Mehta, 2014). Wiberg et al. (1994) demonstrated the need to
81 account for small-scale stratigraphy to represent bed armoring for a non-cohesive model, and did so via a layered bed model
82 that kept track of changes to sediment-bed grain-size distribution in response to cycles of erosion and deposition. Bed layers
83 have been used to represent temporal changes to bed erodibility for fine-grained sediment, for example by using an age
84 model for the bed (HydroQual, 2004). Biodiffusion may alter stratigraphy, and there are many 1DV models that treat



85 diffusive mass flux of sediment and reactive constituents in the bed, mostly motivated by water-quality and geochemical
86 concerns (e.g., Boudreau, 1997; DiToro, 2001; Winterwerp and van Kesteren, 2004). Several regional-scale circulation and
87 sediment-transport models treat sediment stratigraphy, including ECOMSED (HydroQual, 2004), ROMS/CSTMS (Warner
88 et al., 2008), Delft3D (van der Wegen et al., 2011), FVCOM, TELEMAC/SISYPHE (Villaret et al., 2007; Tassi and Villaret,
89 2014), and some have unpublished treatments for cohesive processes. Sanford (2008) pioneered an approach where the
90 critical shear stress for each bed layer was nudged toward an assumed equilibrium value, and the critical stress for erosion of
91 the surface layer alternately became smaller or larger in response to deposition and erosion. We have combined the approach
92 of Sanford et al. (2008) with bioturbative mixing to represent depth-dependent changes in erodibility. This approach has
93 been implemented in the cohesive bed stratigraphy algorithm in ROMS (described here) and applied by Rinehimer et al.
94 (2008), Butman et al. (2014), and Fall et al. (2014).

95 **1.3 Goals of the Model**

96 Our goal in developing and refining sediment dynamics in ROMS is to produce an open-source community model
97 framework useful for research and management that combines cohesive and non-cohesive behavior and is suitable for
98 simulating sediment transport, stratigraphic evolution, and morphologic change. Our goal is to develop methods that can be
99 implemented within coastal and estuarine models for application at regional scales, i.e. domains of 10s to 100s of km² with
100 grid elements of 10 – 10,000 m² and the ability to resolve time scales ranging from minutes to decades.

101 **1.4 Objectives and Outline of the Paper**

102 The behavior of non-cohesive sediment (sand) in ROMS was described by Warner et al. (2008). ROMS also includes several
103 biogeochemical modules (Fasham et al., 1990; Fennel et al., 2006). New components have since been added, including
104 spectral irradiance and seagrass growth model (del Barrio et al., 2014) and a model for treating the effects of submerged
105 aquatic vegetation on waves and currents (Beudin et al., 2017). The present paper describes new components that model
106 processes associated with cohesive sediment (mud) and mixtures of sand and mud. These include aggregation and
107 disaggregation of flocs in the water column, sediment exchange with a cohesive bed where erosion is limited by a bulk
108 critical shear stress parameter that increases with burial depth, and tracking stratigraphic changes in response to deposition,
109 erosion, and bioturbative mixing. Our goal is to demonstrate that the algorithms reproduce some of the important behaviors
110 that distinguish cohesive sedimentary environments from sandy ones, and to demonstrate their utility for modeling muddy
111 environments. The model processes are presented and discussed in Section 2. Additional details of the model implementation
112 and their use in ROMS are presented in the Supplement. Examples of model behavior are presented in Section 3, and a
113 realistic application in the York River Estuary is presented in Section 4. Discussion and Conclusions are in Sections 5 and 6.



114 **2 Model Processes**

115 Flocculation is represented as a local process of aggregation and disaggregation that moves mass among the flocc classes
116 within each model grid cell during a ROMS baroclinic time step. ROMS uses a split time step scheme that integrates over
117 several (ca. 20) depth-averaged (barotropic) time steps before the depth-dependent baroclinic equations are integrated
118 (Shchepetkin and McWilliams, 2005). Subsequent advection and mixing of flocc particles is performed along with other
119 tracers (heat, salt, sand, biogeochemical constituents). The water column is coupled with the sediment bed via depositional
120 fluxes determined by near-bed concentrations, settling velocities, and threshold shear stresses; and via erosional fluxes
121 determined by bottom shear stresses, bulk and particle critical shear stresses for erosion, and sediment availability in the top,
122 active layer (Warner et al., 2008). The distribution of mass among the cohesive classes can change in the bed as floccs are
123 converted to denser aggregates. Deposition and erosion affect the mass of sediment classes in the stratigraphic record, which
124 can also be changed by bioturbative mixing and a heuristic model of erodibility as a function of time and sediment depth.
125 Each of these processes is described below.

126 **2.1 Properties of Sediment, Seafloor, and Seabed**

127 ROMS accounts for two distinct types of sediment: non-cohesive sediment (e.g., sand) and cohesive sediment (e.g., mud).
128 The general framework used to represent sediment and the seabed is unchanged from Warner et al. (2008), except that the
129 expanded model requires additional variables to allow for both cohesive and non-cohesive classes. The number of sediment
130 classes is presently limited to twenty-two of each type by the input/output formats, but is otherwise only constrained by
131 computational resources. Each class must be classified as either non-cohesive or cohesive, and at least one class of one type
132 is required for sediment-transport modeling. Each class is associated with properties (diameter, density, critical shear stresses
133 for erosion and deposition, settling velocity) that are specified as input and remain constant throughout the model
134 calculations. Seafloor properties that describe the condition of the sediment surface are stored with spatial dimensions that
135 correspond to the horizontal model domain. Seafloor properties include representative values (geometric means) of sediment
136 properties in the top layer, including grain size, critical shear stress for erosion, settling velocity, and density; and properties
137 of the sediment surface, such as ripple height, ripple wavelength, and bottom roughness. Seabed properties (i.e. stratigraphy)
138 are tracked at each horizontal location and in each layer in the bed. The number of layers used to represent seabed properties
139 is specified as input and remains constant throughout the model run. The mass of each sediment class, bulk porosity, and
140 average sediment age is stored for each bed layer. The layer thickness, which is calculated from porosity and the mass and
141 sediment density for each class is stored for convenience, as is the depth to the bottom of each layer. Additional information
142 for bulk critical shear stress is stored if the cohesive sediment formulation is being used.



143 2.2 Floc Model

144 We implemented the floc model FLOCMOD (Verney et al., 2011) in ROMS to model changes in settling velocity and
 145 particle size caused by aggregation and disaggregation. Flocculation is represented as a local process that moves mass among
 146 the floc classes within each model grid cell during a ROMS baroclinic time step. Subsequent advection and mixing of floc
 147 particles is performed along with other tracers (heat, salt, sand, biogeochemical constituents). FLOCMOD is a population
 148 model (Smoluchowski, 1917) based on a finite number of size classes with representative floc diameters D_f (m). The model
 149 requires a relationship between floc size and floc density ρ_f (kg/m^3) that is related to the primary disaggregated particle
 150 diameter D_p (m) and density ρ_s (kg/m^3) through a fractal dimension n_f (dimensionless; Kranenburg, 1994) according to

$$151 \quad \rho_f = \rho_w + (\rho_s - \rho_w) \left(\frac{D_f}{D_p} \right)^{n_f - 3} \quad (1)$$

152 where ρ_w (kg/m^3) is the density of the interstitial water in the flocs. The fractal dimension for natural flocs is typically close
 153 to 2.1 (Tambo and Watanabe, 1979; Kranenburg, 1994). Floc densities increase as n_f increases, and at $n_f = 3$, the flocs are
 154 solid particles with $\rho_f = \rho_s$. All cohesive sediment classes are treated as flocs when the floc model is invoked, and the
 155 processes of aggregation and disaggregation can shift mass of suspended sediment from one class to another. The floc model
 156 is formulated as a Lagrangian process that takes place within a model cell over a baroclinic model time step while
 157 conserving suspended mass in that cell, similar to the way that reaction terms are included in biogeochemical models (for
 158 example, Fennel et al., 2006). FLOCMOD simulates aggregation from two-particle collisions caused by either shear or
 159 differential settling, and disaggregation caused by turbulence shear and/or collisions. The rate of change in the number
 160 concentration $N(k)$ (m^{-3}) of particles in the k^{th} floc class is controlled by a coupled set of k of differential equations

$$161 \quad \frac{dN(k)}{dt} = G_a(k) + G_{bs}(k) + G_{bc}(k) - L_a(k) - L_{bs}(k) - L_{bc}(k) \quad (2)$$

162 where G and L terms ($\text{m}^{-3}\text{s}^{-1}$) represent gain and loss of mass by the three processes denoted by subscripts: a (aggregation),
 163 bs (breakup caused by shear), and bc (breakup caused by collisions). Equations 2 are integrated explicitly using adjustable
 164 time steps that may be as long as the baroclinic model time step, but are decreased automatically when necessary to ensure
 165 stability and maintain positive particle number concentrations. Particle number concentrations $N(k)$ are related to suspended
 166 mass concentrations $C_m(k)$ (kg/m^3) via the volume and density of individual flocs. The aggregation and disaggregation terms
 167 (Verney et al., 2011) both depend on local rates of turbulence shear, which are calculated from the turbulence submodel in
 168 ROMS. Details of these processes are described in the Supplement.



169 The floc model introduces several parameters (see Supplement), some of which have been evaluated by Verney et al. (2011).
170 These parameters are specified by the user. The equilibrium floc size depends on the ratio of aggregation to breakup
171 parameters, and the rate of floc formation and destruction depends on their magnitudes (Winterwerp, 1999; 2002). The
172 diameter, settling velocity, density, critical stress for erosion, and critical stress for deposition (described below) are required
173 inputs for each sediment class, both cohesive and non-cohesive (see Supplement). The present implementation requires a
174 fractal relationship between floc diameter and floc density (Kranenburg, 1994), and we have assumed a Stokes settling
175 velocity. Alternative relationships between diameter and settling velocity, such as modified Stokes formula (e.g.,
176 Winterwerp, 2002; Winterwerp et al., 2002; Winterwerp et al., 2007; Droppo et al., 2005; Khelifa and Hill, 2006), could be
177 used by adjusting input parameters, but alternative relationships between diameter and floc density (Khelifa and Hill, 2006;
178 Nguyen and Chua, 2011) would require changes to the aggregation and disaggregation terms in FLOCMOD.

179 2.2.1. Fluxes into the bed – Critical shear stress for deposition

180 The settling flux of flocs (and all other size classes) into the bed (deposition) over a time step is calculated as $w_{s,k} \rho_k C_{v,k} \Delta t$
181 (kg m^{-2} , where $w_{s,k}$ (m/s), ρ_k (kg/m^3), and $C_{v,k}$ (m^3/m^3) are settling velocities, floc (or particle) densities, and volume
182 concentrations for the k th size class in the bottom-most water-column layer, respectively, and Δt (s) is the baroclinic time
183 step. An optional critical shear stress for deposition (τ_d ; Pa; Krone, 1962; Whitehouse et al., 2000; Mehta, 2014) has been
184 implemented for cohesive sediment. Deposition in our model is zero when the bottom stress τ_b (Pa) is greater than τ_d . When
185 τ_b is less than τ_d , deposition increases linearly as τ_b decreases toward zero, behavior we call linear depositional flux
186 (Whitehouse et al., 2000; see Supplement). A simpler alternative is to assume a full settling flux when $\tau_b < \tau_d$, which we call
187 constant depositional flux, and which we have implemented as an option. According to Whitehouse et al. (2000), τ_d is
188 typically about half the magnitude of the critical shear stress for erosion τ_c , but is unrelated to that value. Mehta (2014,
189 Equation 9.83) suggested a relationship between τ_d for larger particles, using τ_d values for the smallest particles in
190 suspension and the ratio of diameters raised to an exponent that depends on sediment properties (see Supplement), citing
191 Letter (2009) and Letter and Mehta (2011). The effect of a critical shear stress for deposition is to keep sediment in
192 suspension in the bottom layer. This results in more material transported as suspended sediment and, for flocs, allows
193 aggregation and disaggregation processes to continue.

194 2.2.2. Changes in floc size distribution within the bed

195 Changes in the size-class distribution of flocs are expected once they have been incorporated into the seabed, in contrast to
196 non-cohesive particles that retain their properties during cycles of erosion and deposition. For example, it seems unlikely that
197 large, low-density flocs can be buried and later resuspended intact, and limited published observations suggest that material



198 deposited as flocs can be eroded as denser, more angular aggregates (Stone et al., 2008). However, we find little guidance for
199 constraining this process. We therefore have implemented deflocculation, a simple process that stipulates an equilibrium
200 cohesive size-class distribution and an associated relaxation time scale. The time-varying size-class distribution in the bed
201 tends toward the user-specified equilibrium distribution while conserving mass (see Supplement). This influences the
202 amount of material in classes that are available for resuspension when a cohesive bed is eroded. Example cases presented
203 below demonstrate the effect of this process and the associated time scale on floc distributions both in the bed and in the
204 water column.

205 **2.3 Stratigraphy**

206 Stratigraphy serves two functions in the model as conditions change and sediment is added or removed from the bed: (1) to
207 represent the mixture of sediment available at the sediment-water interface for use in bedload transport, sediment
208 resuspension, and roughness calculations; and (2) to record the depositional history of sediment. Bookkeeping methods for
209 tracking and recording stratigraphy must conserve sediment mass and must accurately record and preserve age, porosity, and
210 other bulk properties that apply to each layer. Ideally, a layer could be produced for each time step in which deposition
211 occurs, and a layer could be removed when cumulative erosion exceeds layer thickness. In practice, the design of many
212 models is subject to computational constraints that limit resolution to a finite and relatively small number of layers. In
213 ROMS, this number is declared at the beginning of the model run and cannot change. Thus, when deposition requires a new
214 layer, or when erosion removes a layer, other layers must be split or merged so that the total number of layers remains
215 unchanged. Where and when this is done determines the fidelity and utility of the modeled stratigraphic record. Some
216 models have used a constant layer thickness (Harris and Wiberg, 2001); others (for example, ECOMSED) define layers as
217 isochrons deposited within a fixed time interval (HydroQual, Inc., 2004). Our approach is most similar to that described by
218 Le Hir et al. (2011) in that we allow mixing of deposited material into the top layer, and require a minimum thickness of
219 newly formed layers, merging the bottom layers when a new layer is formed. Likewise, the bottom layer is split when
220 erosion or thickening of the active layer, discussed below, reduces the number of layers. The sequence of layer calculations
221 is described in detail in the Supplement.

222 A key component of the bed model is the active layer (Hirano, 1971), which is the thin (usually mm-scale), top-most layer of
223 the seabed that participates in exchanges of sediment with the overlying water. During each model time step, deposition and
224 erosion may contribute or remove mass from the active layer. Any stratigraphy in the active layer is lost by instantaneous
225 mixing (Merkel and Klopmann, 2012), but this is consistent with the original concept of Hirano (1971) and the need to
226 represent the spatially averaged surface sediment properties in a grid cell that represents a heterogeneous seabed. The
227 thickness of the active layer in ROMS scales with excess shear stress (Harris and Wiberg, 1997; Warner et al., 2008) and is
228 at least a few median grain diameters thick (Harris and Wiberg, 1997; see Supplement).



2.4 Bulk Critical Shear Stress for Erosion for Cohesive Sediment

An important difference between cohesive and non-cohesive sediment behavior is that the erodibility of cohesive sediment is treated primarily as a bulk property of the bed, whereas the erodibility of non-cohesive sediment is treated as the property of individual sediment classes. The erodibility of cohesive sediment often decreases with depth in the bed, resulting in depth-limited erosion (Type 1 behavior according to Sanford and Maa, 2001). When the cohesive bed module is used, the erodibility of cohesive beds depends on the bulk critical shear stress for erosion τ_{cb} (Pa), which is a property of the bed layer, not individual sediment classes, and generally increases with depth in the bed. It also changes with time through swelling and consolidation and, in the uppermost layer, is affected by erosion and deposition. The cohesive bed model tracks these changes by updating profiles of τ_{cb} at each grid point during each baroclinic timestep.

There is no generally accepted physically based model for determining τ_{cb} from bed properties such as particle size, mineralogy, and porosity. We adopted Sanford's (2008) heuristic approach based on the concept that the bulk critical shear stress profile tends toward an equilibrium profile that depends on depth in the seabed (Figure 1) and must be determined a priori. Erosion-chamber measurements (Sanford, 2008; Rinehimer et al., 2008; Dickhudt et al., 2009; Dickhudt et al., 2011; Butman et al., 2014) have been used to define equilibrium bulk critical shear stress profiles $\tau_{cb,eq}$ in terms of an exponential profile defined by a slope and offset.

$$\tau_{cb,eq} = a \exp \left[\left(\ln(z_\rho) - offset \right) / slope \right] \quad (3)$$

where z_ρ (kg/m²) is mass depth, the cumulative dry mass of sediment overlying a given depth in the bed. In Equation 3, *offset* and *slope* have units of ln(kg/m²), and $a = 1 \text{ Pa kg}^{-1} \text{ m}^2$ is a dummy coefficient that produces the correct units of critical shear stress. The mass depth at the bottom of each model layer k is calculated as

$$z_\rho(k) = \sum_k \sum_i f_{i,k} \rho_i \Delta z_k \quad (4)$$

where the summations are computed over the k bed layers and i sediment classes, f_i (dimensionless) is the fractional amount of sediment class i , ρ_i (kg/m³) is particle density in class i , and Δz_k (m) is the thickness of layer k . Equation 3 can be written in terms of the power-law fits to erosion-chamber measurements presented by Dickhudt (2008) and Rinehimer et al. (2008; see Supplement). The instantaneous bulk critical shear stress profile is nudged over time scale T_c or T_s (s) toward the equilibrium profile to represent the effects of consolidation or swelling following perturbations caused by erosion or deposition. T_c is the time scale for consolidation and is applied when the instantaneous profile is more erodible than the



255 equilibrium value, while T_s is the time scale for swelling and is applied when the instantaneous profile is less erodible than
256 the equilibrium value. The consolidation time scale is usually chosen to be much shorter than the one associated with
257 swelling (Sanford, 2008). New sediment deposited to the surface layer is assigned a bulk critical shear stress that may either
258 be (1) held constant at a low value (Rinehimer et al. 2008), or (2) set at the instantaneous bed shear stress of the flow.

259 2.5 Mixed Sediment

260 Mixed-sediment processes occur when both cohesive and non-cohesive sediment are present, and are typically sensitive to
261 the proportion of mud. Beds with very low mud content ($<3\%$; Mitchener and Torfs, 1996) behave as non-cohesive
262 sediment: erodibility is determined by particle critical shear stress, which is an intrinsic characteristic of each particle class.
263 Non-cohesive beds may be winnowed and armored by selective erosion of the finer fraction. In contrast, beds with more than
264 3% to 15-30% (Mitchener and Torfs, 1996; Panagiotopoulos et al., 1997, van Ledden et al., 2004; Jacobs et al, 2011) mud
265 content behave according to bulk properties that, in reality, depend on porosity, mineralogy, organic content, age, burial
266 depth, etc., but that, in the model, are characterized by the bulk critical shear stress for erosion. Mixed beds in the model
267 have low to moderate mud content (3% to 30%, subject to user specification) and their critical shear stress in the model is a
268 weighted combination of cohesive and non-cohesive values determined by the cohesive-behavior parameter P_c , which ranges
269 from 0 (non-cohesive) to 1 (cohesive; see Supplement). This approach allows fine material (e.g., clay) to be easily
270 resuspended when P_c is low and only a small fraction of mud is present in an otherwise sandy bed, and it limits the flux to
271 the amount available in the active mixed layer. It also allows non-cohesive silt or fine sand embedded in an otherwise muddy
272 bed to be resuspended during bulk erosion events when P_c is high, and it provides a simple and smooth transition between
273 these behaviors. The thickness of the active mixed layer is calculated as the thicker of the cohesive and non-cohesive
274 estimates. Figure 2 illustrates mixed-bed behavior as the mud (in this case, clay-sized) fraction f_c increases for a constant
275 bottom stress of 0.12 Pa. At low f_c , P_c is zero (Figure 2a), and clay and silt are easily eroded (high relative flux rates out of
276 the bed; Figure 2c) because the particle critical shear stress for non-cohesive behavior of these fine particles is low (Figure
277 2b). The relative flux rates in Figure 2b are normalized by the fractional amount of each class and the erosion-rate
278 coefficient; the actual erosional fluxes for clay content would be low at $P_c = 0$ because of the low clay content in the bed. As
279 f_c increases and the bed becomes more cohesive, relative erosion flux rates decline. When f_c exceeds a critical value (0.2 in
280 the example shown in Figure 2), the bed is completely cohesive and erosion fluxes are determined by bulk critical shear
281 stress for erosion of cohesive sediment τ_{cb} .



282 2.6 Bed Mixing and Stratigraphy

283 Mixing of bed properties in sediment can be caused by benthic fauna (ingestion, defecation, or motion such as burrowing) or
284 circulation of porewater, and tends to smooth gradients in stratigraphy and move material vertically in sediment. The model
285 (e.g., Boudreau, 1997) assumes that mixing is a one-dimensional vertical diffusive process and neglects non-local and lateral
286 mixing processes:

287

288

$$\frac{\partial C_v}{\partial t} = \frac{\partial}{\partial z} \left(D_b \frac{\partial C_v}{\partial z} \right) \quad (5)$$

289 where C_v is the volume concentration of a conservative property (e.g., fractional concentration of sediment classes or
290 porosity), D_b is a (bio)diffusion coefficient (m^2/s) that may vary with depth in the bed (see below), and z (m) is depth in the
291 bed (zero at the sediment-water interface, positive downward). We have discretized equation (5) using the varying bed
292 thicknesses and solve it at each baroclinic time step using an implicit method that is stable and accurate (See Supplement).

293 Biodiffusivity is generally expected to decrease with depth in the sediment (Swift et al., 1994; 1996), but is often assumed to
294 be uniform near the sediment-water interface. The typical depth of uniform mixing, based on worldwide estimates using
295 radionuclide profiles from cores, is 9.8 ± 4.5 cm (Boudreau, 1994). Rates of biodiffusion estimated from profiles of excess
296 ^{234}Th on a muddy mid-shelf deposit off Palos Verdes (California, USA) varied from ~ 2 cm^2/yr to ~ 80 cm^2/yr (Wheatcroft
297 and Martin, 1996; Sherwood et al., 2002) and values from the literature range from 0.01 – 100 cm^2/yr (Boudreau, 1997;
298 Lecroart et al., 2010). The depth-dependent biodiffusion rate profile in the model must be specified for each horizontal grid
299 cell using a generalized shape described in the Supplement.

300 Representation of seabed properties, i.e. the stratigraphy, has been modified slightly from the framework presented in
301 Warner et al. (2008). The revised bed model gives the user latitude to control the resolution of the bed model through the
302 choice of new layer thickness and the number of bed layers, and avoids the mixing described by Merkel and Klopmann
303 (2012). The bookkeeping for bed layers is detailed in the Supplement. The main differences from previous versions of the
304 model (Warner et al., 2008) are the treatments of the second layer (immediately below the active layer) and the bottom layer.
305 During deposition, the new algorithm prevents the second layer from becoming thicker than a user-specified value, which
306 results in thinner layers that can record changes in sediment composition inherited from the active layer as materials settle.
307 During erosion, the new algorithm splits off only a small portion of the bottom layer to create a new layer. This limits the
308 influence of the initial stratigraphy specified for the bottom layer and confines blurring of the stratigraphic record to the



309 bottommost layers. Our tests indicate the new approach provides a more informative record of stratigraphic changes.
310 Moriarty et al. (2017) used a similar approach to bed stratigraphy to preserve spatial gradients in sediment biogeochemistry.

311 **3 Demonstration Cases**

312 The following cases demonstrate the cohesive-sediment processes included in ROMS, explore model sensitivity to
313 parameters, and provide candidates for inter-model comparisons.

314 **3.1 Floc Model**

315 Tests using a quasi one-dimensional vertical implementation of ROMS were conducted to verify that the floc model was
316 implemented correctly and to gain some insight into model behavior under typical coastal conditions.

317 **3.1.1 Comparison with laboratory experiments**

318 Verney et al. (2011) compared results from FLOCMOD with a laboratory experiment of tidal-cycle variation in shear rate G .
319 We performed the same simulations in ROMS by initializing with the same floc model parameters and specified $G(t)$,
320 ranging from $G=0 \text{ s}^{-1}$ at slack tide to $G=12 \text{ s}^{-1}$ at peak flow. The model was run with 15 cohesive classes (instead of the 100
321 classes in the reference FLOCMOD experiment). The class sizes were log-spaced between 4 and 1500 μm with floc densities
322 derived from Equation 1 using $n_f = 1.9$. The initial suspended-sediment concentration was 0.093 kg/m^3 in the 120- μm class.
323 Our results (Figure 3a) matched the cycles of floc diameter variation caused by aggregation (low G) and breakup (high G)
324 shown in Figure 7 of Verney et al. (2011), with a 24- μm root-mean square (rms) difference from observations in mass-
325 weighted mean diameter.

326 We also compared our ROMS FLOCMOD implementation with laboratory experiments of the growth and breakup of flocs
327 performed by Keyvani and Strom (2014) who applied cycles of $G=15 \text{ s}^{-1}$ that caused floc growth followed by long periods
328 (15 h) of very strong turbulent shear rates ($G=400 \text{ s}^{-1}$) that caused disaggregation. We simulated the first cycle of floc
329 formation using the size classes, fractal dimension, and concentrations provided by Keyvani and Strom (2014), but varying
330 the aggregation parameter α and the breakup parameter β that determine the final equilibrium diameter. Our model results
331 with $\alpha=0.1$ and $\beta=0.0135$ (Figure 3b) reproduced the observations with higher skill than the simple model used in their
332 study. The same final diameter was obtained with $\alpha=0.45$ and $\beta=0.06$, but the equilibrium was attained more quickly than
333 observed.

334 These comparisons with laboratory results indicated that our implementation of FLOCMOD in ROMS was correct and
335 demonstrated that the model has useful skill in representing floc dynamics



336 3.1.2. Comparison to equilibrium floc size

337 Simulations were conducted to further evaluate the ROMS implementation of FLOCMOD by comparing modeled
 338 equilibrium floc sizes to equilibrium floc sizes predicted by Winterwerp (2006). He argued that, in steady conditions,
 339 equilibrium floc sizes are determined by the fractal dimension n_f , ratio of aggregation rates and breakup rates, concentration
 340 C (kg/m^3), and turbulence shear rate G (s^{-1}). The equilibrium median floc size D_{50} (m) is given by

$$341 \quad D_{50} = D_p + \frac{k_A}{k_B} \frac{C}{\sqrt{G}} \quad (6)$$

342 where k_A and k_B are aggregation and breakup coefficients, respectively (Winterwerp, 1998). The units of k_A and k_B depend on
 343 fractal dimensions, but the ratio has units of $\text{m}^4\text{kg}^{-1}\text{s}^{-1/2}$. We compared our FLOCMOD results with this theoretical
 344 relationship by running cases with steady conditions, $n_f = 2$, for a range of concentrations ($C = 0.1$ to $10 \text{ kg}/\text{m}^3$), a range of
 345 shear rates ($G = 0.025$ to 100 s^{-1}), and several combinations of aggregation and breakup parameters α and β . The results show
 346 that equilibrium floc size increases with concentration and decreases with turbulence shear rate, as expected (Figure 3c).
 347 Equilibrium diameter is strongly controlled by concentration, and turbulence is more effective at reducing average diameter
 348 at lower concentrations. The slope of the relationship between the equilibrium diameter and C / \sqrt{G} varies with the ratio of
 349 aggregation to breakup. Winterwerp (1998) suggested a slope of about $4 \times 10^3 \text{ m}^4\text{kg}^{-1}\text{s}^{-1/2}$. Figure 3c demonstrates that a range
 350 of slopes can be obtained by varying the ratio α/β . The model reproduced the linear response predicted by Winterwerp
 351 (1998) except near the largest sizes, where our upper limit in floc class size ($5000 \mu\text{m}$) distorted the statistics. Although not
 352 shown in Figure 3c, the floc populations evolved at different rates, depending on α and β , as indicated in Figure 3b.

353

354 3.1.3. Evolution to steady state

355 Steady, uniform flow is a conceptually simple model test that demonstrates the hydrodynamics linking vertical profiles of
 356 flow, evolution of the turbulent boundary layer, and bottom drag. The addition of floc dynamics creates a complicated and
 357 instructive test case. This simulation, forced by a constant sea-surface slope, is similar to the steady flow test examined by
 358 Winterwerp (2002, section 4.8.1), and produces a linear Reynolds-stress profile increasing from zero at the surface to
 359 $\tau_b = -\rho_w g h ds/dx$ at the seabed, where τ_b (Pa) is bottom shear stress, g (m/s^2) is gravitational acceleration, h (m) is water
 360 depth, and ds/dx (m/m) is sea-surface slope. The flow develops a logarithmic velocity profile $u = (u_* / \kappa) \ln(z / z_o)$, where
 361 u (m/s) is velocity in the x direction, $u_* = \sqrt{\tau_b / \rho_w}$ is shear velocity (m/s), $\kappa = 0.41$ (dimensionless) is von Kármán's



362 constant, z (m) is elevation above the bed, and z_0 (m) is the bottom roughness length. The final flow velocity near the
363 surface is about 0.6 m/s. When non-cohesive sediment is added (and erosion and deposition are set to zero), the suspended
364 sediment concentrations for each size class evolve into Rouse-like profiles where, at each elevation, downward settling is
365 balanced by upward diffusion. The addition of floc dynamics complicates the situation, because aggregation creates larger
366 flocs with higher settling velocities. The larger flocs tend to settle into regions of higher shear and higher concentration,
367 where the higher shear tends to break them into smaller flocs but the higher concentrations enhance aggregation. The size
368 distribution, settling velocity, concentration, shear, and turbulent diffusion evolve to a steady state under a dynamic balance.
369 The resulting profiles of concentration and mass-weighted average size and settling velocity are sensitive to both floc model
370 parameters and modeled physical conditions (water depth, bottom stress, turbulence model, total sediment in suspension).

371 We demonstrate this process using 22 floc classes with logarithmically spaced diameters ranging from 4 to 5000 μm (Figure
372 4). The initial vertical concentration profile was uniform at 0.2 kg/m^3 , all in the 8- μm class. The model started from rest, and
373 the initial response was slow particle settling in the nearly inviscid flow: concentrations, floc sizes, and settling velocities all
374 decreased near the surface (Figures 4a, b, and c). As the flow accelerated in the first two hours, turbulence generated by
375 shear at the bottom began to mix upward in the water column, diffusing settled material higher and facilitating collisions and
376 aggregation among flocs. Between hours 3 and 4, settling was enhanced by these newly formed larger flocs, as is apparent in
377 increases in average diameter and settling velocities, and reduced concentrations near the surface. Equilibrium was nearly
378 established by about hour 5. At the end of the model run, the total concentration profile decreased exponentially with
379 elevation (Figure 4d and 4g), but average size and settling velocities both decreased markedly in the bottom meter (Figures
380 4e and 4f), reflecting shear disaggregation that lead to increases in smaller flocs near the bottom (Figure 4g).

381 The time scales to achieve equilibrium in this simulation are comparable to tidal time scales, suggesting equilibrium is
382 unlikely in the real world, where forcing is time dependent and bottom conditions are spatially variable. The final condition
383 is sensitive to flow forcing, initial concentrations, and floc parameters. For example, when concentrations are higher, or
384 when the disaggregation parameter is increased (making the flocs more fragile), bottom-generated shear causes
385 disaggregation higher into the water column, and mid-depth maxima in diameter and settling velocity evolve. This steady
386 flow simulation is useful as both a standard test case and a reminder of the complexity of floc processes, even when the
387 hydrodynamics are relatively simple.

388 3.1.4. Settling fluxes

389 Interaction with the bed influences the evolution of the floc population in the water column by providing sources or sinks in
390 various size classes. We have experimented with several sediment-flux conditions from the water column to the seabed,



391 including settling fluxes, zero fluxes, and fluxes modulated by threshold stresses for deposition. Settling fluxes calculated as
392 $w_k \rho_k C_k \Delta t$ summed over each class k , is the default method used for non-cohesive sediment. Zero-flux boundary conditions
393 essentially treat the bottom water-column cell as a fluff layer, allowing flocs to accumulate by settling or mix out by
394 diffusion. Floc dynamics continue to operate in this layer, so the size distributions change with concentration and stress.
395 Settling fluxes modulated by stress thresholds for deposition allow flocs to deposit only under relatively quiescent
396 conditions. The model framework provides a variety of choices described in the Supplement, each with implications that
397 must be assessed in the context of the problem at hand. As expected, the conditions that reduced settling into the bed resulted
398 in higher sediment concentrations in the bottommost water-column layer and allowed for floc breakup by the enhanced near-
399 bottom turbulence.

400 3.1.5. Model sensitivity

401 A wide range of model runs (not presented here) have provided us with a qualitative sense of model performance. Model
402 results respond as expected to physical parameters, such as mean concentration and shear rate (discussed above), as well as
403 primary particle size and fractal dimension. Model results are also sensitive to model configuration, including the number of
404 size classes, the size of vertical grid spacing, and the time step used. Our experience so far confirms that of Verney et al.
405 (2011): a truncated distribution of about seven size classes provides qualitatively useful results, but the choice of size range
406 and size distribution may change the results. The sensitivity to vertical grid resolution is particularly important in the
407 bottommost layer, which has the highest concentrations and highest shear rates. Finer grid spacing near the bottom results in
408 layers with higher shear and higher sediment concentrations, which cause local changes in the equilibrium floc sizes. Model
409 time steps in our floc model tests are short, ranging from 10 to (more typically) 1 s. The adaptive sub-steps for aggregation
410 and disaggregation were limited to a minimum of 0.5 s. At high concentrations ($> 0.2 \text{ kg/m}^3$) and high shear rates, the results
411 sometimes showed numerical instability, probably related to the explicit solution of Equations 2. Replacement of the solver
412 for these equations with a faster and more robust method in the future should improve model stability.

413 3.2 Resuspension

414 Three cases are presented here to demonstrate the evolution of stratigraphy caused by resuspension and subsequent settling
415 of sediment during time-dependent bottom shear stress events. They contrast model calculations using the non-cohesive and
416 mixed-bed routines, and highlight the role of biodiffusion. These were one-dimensional (vertical) cases represented with
417 small ($\sim 5 \times 6$ horizontal $\times 20$ vertical cells), three-dimensional domains with flat bottoms and periodic lateral boundary
418 conditions on all sides. They were forced with time-varying surface wind stress that generated time-dependent horizontal
419 velocities and bottom stress, initialized with zero velocity and zero suspended-sediment concentration, and did not include
420 floc dynamics in the water column.



421 3.2.1 Non-cohesive bed simulation

422 A non-cohesive bed simulation was used to demonstrate the generation and preservation of sand and silt stratigraphy during
423 a resuspension and settling event (Figure 5). The model was forced with two stress events ~ 1.5 d apart and lasting 1.5 d and
424 1 d respectively. Four sediment classes, representing particles with nominal diameters of 4, 30, 62.5, and 140 μm , particle
425 critical shear stresses of 0.05, 0.05, 0.1, and 0.1 Pa, and settling velocities of 0.1, 0.6, 2, and 8 mm s^{-1} were used. Although
426 the diameters of the first two sediment classes corresponded to mud, all sediment classes in this experiment were treated as
427 non-cohesive material. The initial sediment bed contained 41 layers, each 1 mm thick, and each holding equal fractions
428 (25%) of the four sediment classes. New sediment layers were constrained to be no more than 1 mm thick.

429 The first, larger stress event (maximum $\tau_b = 1$ Pa; Figure 5b), eroded 1.2 cm of bed, and recruited additional fine sediment
430 from the active layer, which extended 2 cm below the initial sediment surface (Figure 5d). The finer fractions dominated the
431 suspended sediment in the water column, which contained only a small fraction of the coarsest sand (Figure 5a). When the
432 stress subsided, coarser sediment deposited first, while finer material remained suspended, producing graded bedding above
433 the 2-cm limit of initial disturbance (Figure 5d). The second stress pulse eroded the bed down to 1 cm but only resuspended
434 minimal amounts of the 140- μm sand. Deposition resumed after the second pulse subsided and, at the end of the simulation,
435 some mud remained in the water column (Figure 5a), leaving the bed with net erosion of 5 mm (Figure 5d). The finest
436 material (4 μm) remained mostly in suspension after five days. The final thickness of the bottom five layers was smaller than
437 their initial value (1 mm), because, to maintain a constant number of bed layers, the deepest layer was split each time a
438 surface layer was formed during deposition. The two stress pulses affected sediment texture down to 2 cm. Above this level,
439 almost all of the finest class was winnowed, and remained mostly in suspension while the other classes settled to the bed, so
440 that the upper bed layers developed a fining-upward storm layer. The bottom portion of the storm layer (1 – 2 cm depth) was
441 a lag layer comprised of the two coarsest classes, both because these resisted erosion and because the sand that did erode
442 settled to the bed quickly when shear stress decreased.

443 3.2.2 Mixed bed simulation

444 This case examined the stratigraphic consequences of cohesive behavior resulting from a single bottom-stress event (Figure
445 6). The model configuration was similar to the previous example. The same sediment classes were used, but the two finest (4
446 and 30 μm) were treated as cohesive mud, while the other two remained non-cohesive (sand). The fraction of cohesive
447 sediment ($f_c = 0.5$) exceeded the chosen non-cohesive threshold (f_{nc} threshold = 0.2), so the bed behaved as if it were
448 completely cohesive. The cohesive formulation required the initialization of an equilibrium bulk critical stress profile for
449 erosion $\tau_{ob,eq}(z)$. We chose parameters within the range of sensitivities studied by Rinehimer et al. (2008) and specified an



450 equilibrium profile with a $slope = 2 \ln(\text{kg}/\text{m}^2)$ and an $offset$ of $3.4 \ln(\text{kg}/\text{m}^2)$, with a minimum value of 0.03 Pa and a
451 maximum of 1.5 Pa (dashed magenta line in Figure 6b) and initialized the model with this profile (solid purple line in Figure
452 6b). The time scale for consolidation was set to $T_c = 8$ hours. The swelling time scale was chosen to be 100 times longer than
453 consolidation ($T_s = 33$ days). A time series of bed stress was imposed (Figure 6a), and the bed responded initially by
454 eroding. As the imposed stress waned starting at day 1.8, sediment settled to the bed causing deposition. The initial rapid
455 increase in bottom stress during the first 0.7 days (Figure 6a) exceeded the critical stress of the bed to a depth of 2.4 cm (red
456 line in Figure 6c), causing resuspension and erosion of the top 5 mm of the bed. In this case, the amount of material eroded
457 was limited by the erosion rate coefficient. The equilibrium critical stress profile, which has a static shape, shifted down with
458 the sediment-water interface (compare dashed magenta line in Figures 6b, c). After the initial erosion, the instantaneous
459 critical stress profile tended toward the equilibrium critical stress profile over the slow swelling time scale of 33 days,
460 rendering the bed a little more erodible (compare Figures 6c, d). By day 2.8, the stress had waned and 4 mm of sediment had
461 redeposited (Figure 6d). The equilibrium critical stress profile had shifted upward with the bed surface, causing the
462 instantaneous critical stress to increase over the short compaction time scale. The final instantaneous critical shear stress
463 profile (Figure 6e) had reached the long-term equilibrium everywhere except in the most recent deposits. This case
464 exemplifies the sequence of depth-limited erosion, deposition, and compaction that characterizes the response of mixed and
465 cohesive sediment in the model.

466 3.2.3 Biodiffusion simulations

467 We validated the numerical performance of the biodiffusion algorithms using two analytical test cases with a realistic range
468 of parameters. The implicit numerical solution is unconditionally stable and conserves mass to within 10^{-8} %, but the
469 accuracy depends on time step, gradients in biodiffusivity, and bed thickness. Typical RMS differences in the fractional
470 amount of sediment in a particular class between the numerical solutions and the analytical solutions ranged from 10^{-2} to 10^{-6} .
471 We found that, for modeled beds 5 m thick, solutions improved as layer thickness decreased from 50 to 5 cm, but beyond
472 that, higher resolution did not substantially improve the solution. Even in the worst case, where the numerical solution was
473 off by 1%, it was much more precise than our estimates of biodiffusivity coefficients.

474 Four cases are presented to demonstrate bed mixing (Figure 7). The first two used the same configuration as in the non-
475 cohesive (Figures 5, 7a) and mixed-bed simulations (Figures 6d, 7b). The second two were identical to the mixed-bed case
476 except that biodiffusive mixing was enabled. The biodiffusivity profile used was similar to that proposed for the mid-shelf
477 deposit offshore of Palos Verdes, CA (Sherwood et al., 2002) that had a constant diffusivity D_{bs} from the sediment-water
478 interface down to 2 mm, an exponential decrease between 2 mm and 8 mm, and a linear decrease to zero at 1 cm depth.



479 These two cases differed in their biodiffusion coefficients: a) the first used relatively large biodiffusion coefficients ($D_{bs} =$
480 $10^{-5} \text{ m}^2\text{s}^{-1}$); b) the second used smaller values ($D_{bs} = 10^{-10} \text{ m}^2\text{s}^{-1}$).

481 The resulting stratigraphy after the five-day simulation (Figure 7) indicates that mixing in the case with large biodiffusivity
482 (Figure 7c) tended to smooth all gradients rapidly and only during depositional conditions was the vertical structure of grain
483 size fractions preserved. Some sediment remained in suspension in all four cases, which was reflected in the final bed
484 elevation. The resulting top 1 cm of the bed was always well mixed and the depth of the disturbed sediment at the end of the
485 simulation was deeper (2.5 cm) in this case than in the other simulations. Sediment deeper than 2.5 cm below the surface was
486 undisturbed: it was beyond the reach of erosion, active-layer formation, and biodiffusion. The biodiffusive mixing increased
487 recruitment of fine sediment into the surface active layer during erosion, resulting in increased concentrations in the water
488 column (not shown) compared to the mixed bed case without biodiffusion.

489 The case with a smaller biodiffusion coefficient (Figure 7d) developed stratigraphy intermediate to those cases with large
490 and zero biodiffusion. The depth of disturbed sediment was 2.3 cm and the transition between redeposited sand and mud was
491 smooth with coarse sand being present at the surface of the bed. This gradual size gradation was intermediate to the sharp
492 jump in the fractional distribution between mostly sandy layers and predominantly muddy layers produced in cases that
493 neglected mixing (Figure 7a,b) and the smooth gradient produced by the strong mixing case (Figure 7c).

494 3.3 Estuarine Turbidity Maxima

495 High concentrations of suspended sediment often occur near the salt front in estuaries, forming estuary turbidity maxima
496 (ETM). We present ETM test cases that simulated sediment transport in a two-dimensional (longitudinal and vertical) salt-
497 wedge estuary with tidal and riverine forcing. The cases investigated the formation of cohesive deposits beneath the ETM
498 with and without floc dynamics. The first case, without floc dynamics but with a mixed bed, is presented here. The second
499 case, presented below, adds floc dynamics. The model was forced with a 12-hour tidal oscillation modulated with a 14-day
500 spring-neap cycle. The idealized estuary was 100-km long with a sloping bottom 4 m deep at the head of the estuary and 10
501 m deep at the mouth (Figure 8a). In all cases, the simulations were run for twenty tidal cycles. Two non-cohesive sediment
502 classes (180- and 250- μm diameter) were represented with equal initial bed fractions (50% of each). One cohesive fraction
503 ($37 \mu\text{m}$, $\rho_f = 1200 \text{ kg/m}^3$, $w_s = 0.13 \text{ mm/s}$) was included, with an initial uniform suspended-sediment concentration of 1
504 kg/m^3 . The bed was initialized without any cohesive sediment, so it initially behaved non-cohesively. Later in the simulation,
505 bed behavior became mixed as suspended mud settled and was incorporated into the initially sandy bed. The chosen
506 equilibrium bulk critical shear stress profile (Equation 3) had $\text{slope} = 5 \ln(\text{kg/m}^2)$ and $\text{offset} = 2 \ln(\text{kg/m}^2)$, with a minimum
507 value of 0.05 Pa and a maximum of 2.2 Pa. The time scale for consolidation was set to $T_c=8$ hours (Sanford, 2008;
508 Rinehimer, 2008), and the swelling time scale was set to $T_s=33$ days.



509 During the simulations, salinity and suspended-sediment field evolved into dynamic equilibria that were repeated over
510 consecutive tides. An estuarine turbidity maximum (ETM) developed between 10 km and 60 km from the mouth of the
511 estuary (Figure 8a) in the salt wedge generated by gravitational circulation and tidal straining (Burchard and Baumert, 1998;
512 MacCready and Geyer, 2001). Elevated suspended-sediment concentrations ranging from 0.7 to 2.05 kg/m³ occupied most of
513 the bottom layer and extended to mid-depth. All of the suspended material was in the 37- μ m class (Figure 8a).

514 The second case was identical, except that it included floc dynamics. Fifteen cohesive (floc) classes and the two non-
515 cohesive (sand) classes were included. Floc-class diameters were logarithmically spaced, ranging from 20 to 1500 μ m, with
516 floc densities ranging from 1350 to 1029.3 kg/m³, and settling velocities ranging from 0.078 to 5.31 mm/s, commensurate
517 with Equation 1 with fractal dimension $n_f = 2$. The suspended-sediment concentration field was initialized with a uniform
518 concentration of 1 kg/m³, all in the 37- μ m class. The resulting ETM (Figure 8b) extended farther up-estuary and contained
519 much lower concentrations (0.1 to 0.5 kg/m³ in most of the salt wedge, with a thin layer of higher concentrations (2.1 kg/m³)
520 in the bottom layer (bottom 5% of the water column). The second layer (5 – 10% of the water column) had concentrations
521 about half of the bottom layer. The bed sediment response for the two cases also differed. In the no-floc case, the ETM
522 deposit was slightly thinner, located closer to the mouth, and varied less from slack to flood (Figure 8c). Floc dynamics
523 created large tidal variations in the size of bed material (Figure 8d), which ranged up to 600 μ m as flocs deposited during
524 slack, and decreased to 37 μ m as flocs were resuspended during flood. The behavior in the unflocculated case was less
525 intuitive. Over the course of the simulation, enough fine material accumulated beneath the ETM to cause the bed to behave
526 cohesively, but the top, active layer remained mostly non-cohesive. During flood tide, bottom stresses were sufficient to
527 resuspend the non-cohesive 70 μ m material, leaving the cohesive 37 μ m material on the bed. Thus, in both cases, the bed
528 became finer during period of higher stress, but for different reasons. The two cases highlight the model-dependent changes
529 in location (driven primarily by settling velocities) and size distributions (driven by floc dynamics) of the ETM.

530 We next expanded the numerical experiment, using six floc cases to elucidate the effects of floc dynamics in the idealized
531 estuary (Table 1). The two-dimensional model domain was the same as the ETM case described above. Three types of floc
532 behavior in the seabed were investigated: (1) no changes in size distribution occurred in the bed; (2) the bed deflocculation
533 process was invoked, which nudged all cohesive sediment into the 20- μ m class over a long time scale (50 hours); and (3) the
534 bed deflocculation process was invoked with a short time scale (5 hours). Additionally, three other combinations of
535 aggregation (α) and disaggregation (β) rates were used with the slow deflocculation rate to explore floc processes in the
536 water column (Table 1). The following six metrics were compared at the location of the maximum depth-mean suspended-
537 sediment concentration (SSC): depth-mean SSC; maximum SSC; median size (D_{50}); 12-h mean of the D_{50} ; depth-mean
538 settling velocity w_s ; and depth-mean w_s averaged over a 12-h tidal period (Table 1). The median size and mean settling



539 velocities were weighted by the mass in each class. Also listed in Table 1 are the locus of the maximum deposition, the
540 thickness at that location, and the median size of deposited material at that location.

541 Mean SSC in the ETM did not vary significantly among the floc cases, but the maximum SSC (located lower in the water
542 column) increased when the ratio of aggregation rate / disaggregation rate α / β was higher, which led to larger, faster-
543 settling flocs. Among the four cases (3 – 6) with slow deflocculation rates in the bed, settling velocities, maximum SSC, and
544 floc size covaried. The locus of maximum deposition of ETM material was insensitive to the deflocculation algorithms
545 (cases 1 – 3), and most sensitive to the overall floc rates. The range of ETM locations is listed in Table 6 to highlight the
546 cases where ETM location varied. The case with lowest floc rates (case 5) produced the farthest upriver deposit, with the
547 most variation in the location of the maximum. The case with the highest settling velocities (case 6) produced deposits
548 closest to the estuary mouth. Overall, the simulated ETM was more sensitive to changes in floc parameters than to prescribed
549 behavior of the floc population in the seabed (deflocculation), and the greatest effect of varying floc dynamics was the
550 vertical location of the ETM, which was controlled by floc size and settling velocity.

551 **4 Realistic Application: York River Estuary**

552 This section demonstrates the cohesive sediment bed model in a realistic domain representing the York River, a sub estuary
553 of Chesapeake Bay (Figure 9). Recent modeling efforts have focused on this location as part of a program aimed at exploring
554 links between cohesive sediment behavior, benthic ecology, and light attenuation. As part of this program, colleagues have
555 obtained complementary field observations there, which have been especially focused on the two locations off Gloucester
556 Point and Clay Bank, VA (e.g. Dickhudt et al. 2009, 2011; Cartwright et al. 2013). The implementation presented here is
557 similar to the three-dimensional model developed by Fall et al. (2014) that accounted for circulation, sediment transport, and
558 a cohesive bed. While this model neglects flocculation, information obtained by field observations such as Cartwright et al.
559 (2013) have been consulted for guidance in setting settling velocities of the cohesive particles. The model is run assuming
560 muddy behavior of the bed, and neglecting mixed bed processes, because the majority of sediment transport within the York
561 River channels consists of fine-grained material. We found that it was important to modify the sediment bed layering
562 management scheme, as discussed in section 5 below, to resolve the high gradients in bed erodibility evident in the sediment
563 bed model (i.e. Fall et al 2014) and data (i.e. Dickhudt et al. 2009, 2011).

564 In this implementation, sediment deposited to the bed provided an easily erodible layer with an assumed low critical stress, τ_c
565 = 0.05 Pa. The modeled sediment bed erodibility and suspended-sediment concentrations both were found to be sensitive to
566 parameterization of the equilibrium critical stress profile, and to the consolidation and swelling timescales used (Fall et al.,



2014). Here we present a case similar to that shown by Fall et al. (2014), but that differs mainly in terms of the sediment bed initialization. The equilibrium critical stress profile was chosen as $\tau_{cb,eq} = z_p^{0.62}$ which was a power-law fit of erodibility experiments performed by Dickhudt (2008) on field-collected cores in September 2007 (Rinehimer et al., 2008). Swelling and consolidation timescales of 1 day and 50 days, respectively, were used. Both the porosity ($\phi = 0.9$) and the erosion rate parameter $E_0 = 0.03 \text{ kg}/(\text{m}^2 \text{ s Pa})$ were held constant. A zero-gradient condition was applied for suspended-sediment concentration at the open boundary where the York River meets Chesapeake Bay. Six sediment classes that had settling velocities ranging from 0.032 to 10 mm/s were used. To initialize the seabed, they were distributed in equal fractions throughout the model domain in a 20-layer sediment bed that had a total thickness of 1 m, with all but the bottom layer being thin (0.1 mm). In this way, the model was initialized with a sediment bed that had high vertical resolution (0.1 mm) in the upper ~2 cm, underlain by a thick layer (~1 m) sediment. This created high vertical resolution in the bulk critical shear stress profile near the sediment – water interface, while still providing a fairly large pool of sediment so that erosional locations retained some sediment in the seabed throughout the model run. Bed critical stress was initialized everywhere to be constant (0.05 Pa) with depth, and quickly evolved to the equilibrium critical shear stress profile at the compaction time scale of a few days. The model was run to represent two months using the sixty-year median freshwater flow of $67 \text{ m}^3/\text{s}$ and a spring-neap tidal cycle with 0.2-m neap amplitude and 0.4-m spring amplitude.

The initially uniform bed evolved during the 60-day model run, developing areas of high sediment erodibility along the shoals of the estuary and channel flanks (Figure 10a). In general, sediment was removed from the main channel, which developed reduced erodibility (Figure 10a). At the Gloucester Point site, the initial bed evolved to become less erodible, with a critical shear stress at the seabed that exceeded the equilibrium values specified for the model (Figure 10a). Conversely, at the Clay Bank field site, conditions were variable in space. Sediment deposited on the shoal area, which evolved to enhanced erodibility (Figure 10a). Within the channel, however, the equilibrium critical stress for erosion was often exceeded, resulting in a strongly eroded sediment bed having larger values of critical shear at the sediment surface (Figure 10a). Resuspension and transport also changed the spatial distribution of sediment classes, with the erosional areas retaining only the coarser, faster-settling classes, while depositional areas retained finer-grained, slower-settling particles (Figure 10 b, c). These patterns, with coarse lag layers and reduced erodibility in the channels relative to the shoals, are consistent with the known grain size distributions and properties of the York River Estuary.

5 Discussion

The model algorithms presented here were motivated by the need to improve the representations of sediment dynamics in numerical models of fine-grained and mixed-sediment environments. The improvements were implemented in the COAWST



596 version of ROMS, which provides a framework for realistic two-way nested models with forcing from meteorology and
597 waves. ROMS includes options for several turbulence sub-models (e.g., $k - \varepsilon$, $k - \omega$, Mellor-Yamada) and wave-current
598 bottom-boundary layer sub-models that allow us to calculate fields of shear velocity G . Implementation of FLOCMOD in
599 this framework provides a platform for numerical experiments and real-world applications of a full-featured flocculation model.

600 The primary role of the flocculation model is to simulate the dynamical response of particle settling velocities to spatial and temporal
601 variations in shear and suspended-sediment concentrations. This can also be achieved with simpler and computationally
602 more efficient parameterization in many applications. What are the advantages of the complex and much slower model
603 implemented here? There are several. The flocculation model provides fields of particles with dynamically varying density and
604 number of primary particles, which allow calculation of the acoustic and optical responses of the particle fields. In turn, this
605 allows direct comparison with field measurements of light attenuation, optical backscatterance, and acoustic backscatterance,
606 the de facto proxies for suspended-sediment concentration. This also allows calculation of derived properties in the water
607 column, including light penetration and diver visibility. Finally, the modeled particle properties can be used in geochemical
608 calculations that require estimates of particle radius, porosity, and reactive surface area. Depending on the application, this
609 additional information may justify the computational expense of the flocculation model.

610 The cohesive bed model provides a heuristic but demonstrably useful tool for representing muddy and mixed beds. The
611 cohesive bed framework captures the most important aspects of muddy environment: limitations on erosion caused by
612 increased bed strength with depth in the sediment, and changes toward user-defined equilibrium conditions as deposited (or
613 eroded) beds age. The physical processes of self-compaction and associated changes in porosity and bed strength are not
614 modeled, but the framework of particle-class and bed-layer variables are designed to accommodate a compaction algorithm.
615 The equilibrium profile method implemented here adds little computational expense, but allows the model to represent
616 depth-limited erosion, a key property of many cohesive beds.

617 Modeling stratigraphy effectively is challenging. Although conserving sediment mass among a fixed number of layers is
618 straightforward, it has proven difficult to devise a robust and efficient method that records relevant stratigraphic events in a
619 modeled sediment bed over the wide range of conditions that occur in coastal domains. For both sediment transport and
620 sediment bed geochemistry (i.e. Moriarty et al. 2017), it can be important for the sediment bed model to achieve its highest
621 vertical resolution near the sediment – water interface, but the original ROMS sediment bed model did not meet that goal
622 when the sediment bed was subject to frequent or repeated cycles of erosion. The modifications we have made to the bed-
623 layer management have improved the fidelity with which we can record stratigraphic events in the model layers, particularly
624 at the sediment – water interface. Inclusion of bioturbative mixing is important for environments where biological activity is
625 rapid, compared with sedimentation or physical reworking. Additionally, for problems of sediment geochemistry, it is
626 important to account for mixing of both particulate matter and porewater. Expansion of the ROMS sediment bed model to



627 include diffusive mixing facilitates its use for interdisciplinary problems (i.e. Moriarty et al. 2017). The choice of appropriate
628 mixing parameters remains a challenge, especially when considering the spatial and seasonal heterogeneity of biological
629 activity.

630 Overall, the cohesive and mixed-bed algorithms we have introduced in ROMS provide tools that should be useful for both
631 numerical experimentations and realistic applications for fine-grained, and mixed-bed environments. Our implementation of
632 flocculation, bed consolidation, and bed-mixing modules enhance the utility of the ROMS sediment model for
633 interdisciplinary studies including ecosystem feedbacks (light attenuation, biogeochemistry), and contaminant transport.

634 **6 Conclusion**

635 This paper describes three ways in which the sediment model of Warner et al. (2008) has been enhanced, allowing
636 simulations to be made for non-cohesive, cohesive, and mixed sediment and allowing it to be applied in a wider range of
637 studies. A flocculation model has been added, following Verney et al. (2011). The cohesive bed model developed by Sanford
638 (2008) has been added, allowing the erodibility of the sediment bed to evolve in response to the erosional and depositional
639 history. Mixing between bed layers has been implemented as biodiffusion using a user-specified diffusion coefficient profile.
640 In addition, the sediment bed layering routine has been modified so that bed layers maintain a high resolution near the
641 sediment water interface, as demonstrated by both our idealized and realistic case studies presented here. The paper presents
642 results of model runs that test and demonstrate these new features and to show their application to real-world systems. The
643 authors encourage the coastal modeling community to use, evaluate, and improve upon the new routines.

644 **Code and Data Availability**

645 The algorithms described here have been implemented in ROMS (version 3.6) distributed with the Coupled Ocean
646 Atmosphere Waves Sediment-Transport Modeling System (COAWST, Subversion repository revision number 1179).
647 COAWST is an open-source community modeling system with a Subversion source-control system maintained by John C.
648 Warner (jcwarner@usgs.gov) and distributed under the MIT/X License (Warner et al., 2010). The COAWST distribution
649 files contain source code derived from ROMS, WRF, SWAN, MCT, and SCRIP, along with Matlab code, examples, and a
650 User's Manual.



651 **Supplement Link (supplied by Copernicus)**

652 **Team List**

653 **Author Contribution**

654 C.R. Sherwood and A. Aretxabaleta shared development of the model code and test cases and most of the manuscript
655 preparation. J.P. Rineheimer was an early user of the cohesive bed model and, along with C.K. Harris, developed the York
656 River application. R. Verney graciously supplied his FORTRAN version of FLOCMOD and helped with adaptation for
657 ROMS. B. Ferré contributed to the early development and application of the model. All authors contributed to the final
658 version.

659 **Competing Interests**

660 The authors declare that they have no conflict of interest.

661 **Disclaimer**

662 Use of firm and product names is for descriptive purposes only and does not imply endorsement by the U.S. Government.

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670 Council (grant no. 223259). The model code is implemented in ROMS version 3.6, as distributed with the COAWST
671 modeling system (Subversion repository revision 1179; Warner et al., 2010), and is freely available by request to John C.
672 Warner (jcwarner@usgs.gov) at the U.S. Geological Survey.



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876 **Table**

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Table 1. Characteristics of the estuary turbidity maxima for seven cases under different flocculation conditions.

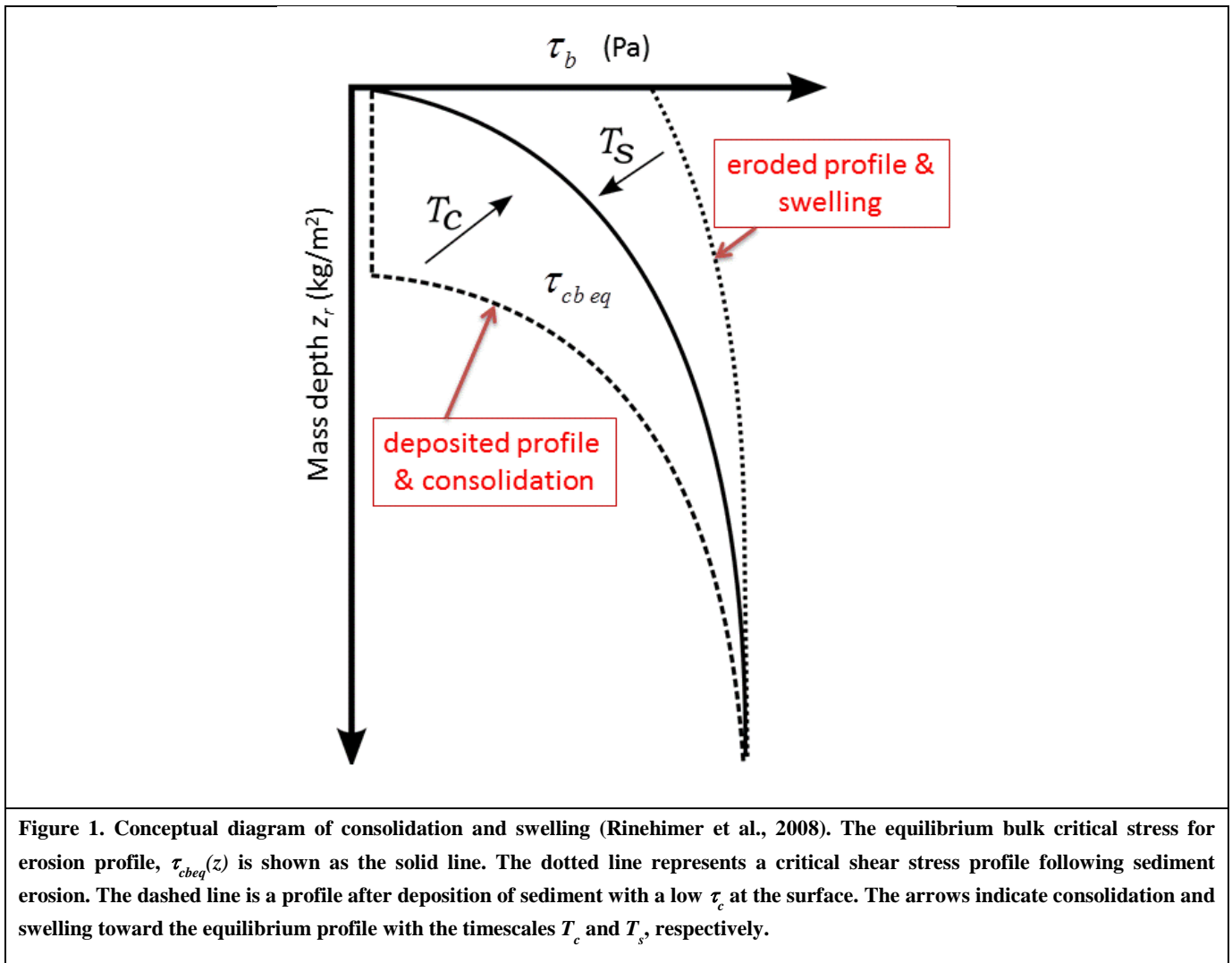
Case	0	1	2	3	4	5	6
	No floccs	$\alpha = 0.35$ $\beta = 0.15$ no defloc	$\alpha = 0.35$ $\beta = 0.15$ defloc=5h	$\alpha = 0.35$ $\beta = 0.15$ defloc=50h	$\alpha = 0.45$ $\beta = 0.10$ defloc=50h	$\alpha = 0.25$ $\beta = 0.20$ defloc=50h	$\alpha = 0.35$ $\beta = 0.34$ defloc=50h
Mean SSC @ maximum (kg/m ³)	1.23	0.46	0.45	0.45	0.45	0.46	0.46
Maximum SSC (kg/m ³)	3.1	3.6	3.7	3.7	4.1	3.2	2.9
D_{50} at SSC maximum (μm)	37	539	529	529	622	426	384
D_{50} at SSC maximum; 12-h mean (μm)	37	255	249	250	325	181	167
w_s at SSC maximum (mm/s)	0.13	1.91	1.87	1.87	2.2	1.51	1.36
w_s at SSC maximum; 12-h mean (mm/s)	0.13	0.90	0.88	0.89	1.15	0.64	0.59
Locus of maximum deposition (km from ocean boundary)	80 ± 30	19 ± 11	18 ± 10	18 ± 11	19 ± 10	79 ± 69	16 ± 6
Maximum deposit thickness (mm)	4.2 ± 5.8	31.6 ± 12.8	25.8 ± 10.1	26.1 ± 10.4	27.1 ± 10.9	5 ± 10.1	25 ± 10.2
Maximum deposit D_{50} (μm)	18.5 ± 0	218 ± 87.1	40.9 ± 71.3	75.5 ± 76.1	92.9 ± 94.2	69.5 ± 89.9	25.4 ± 40.4

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880 **Figures**

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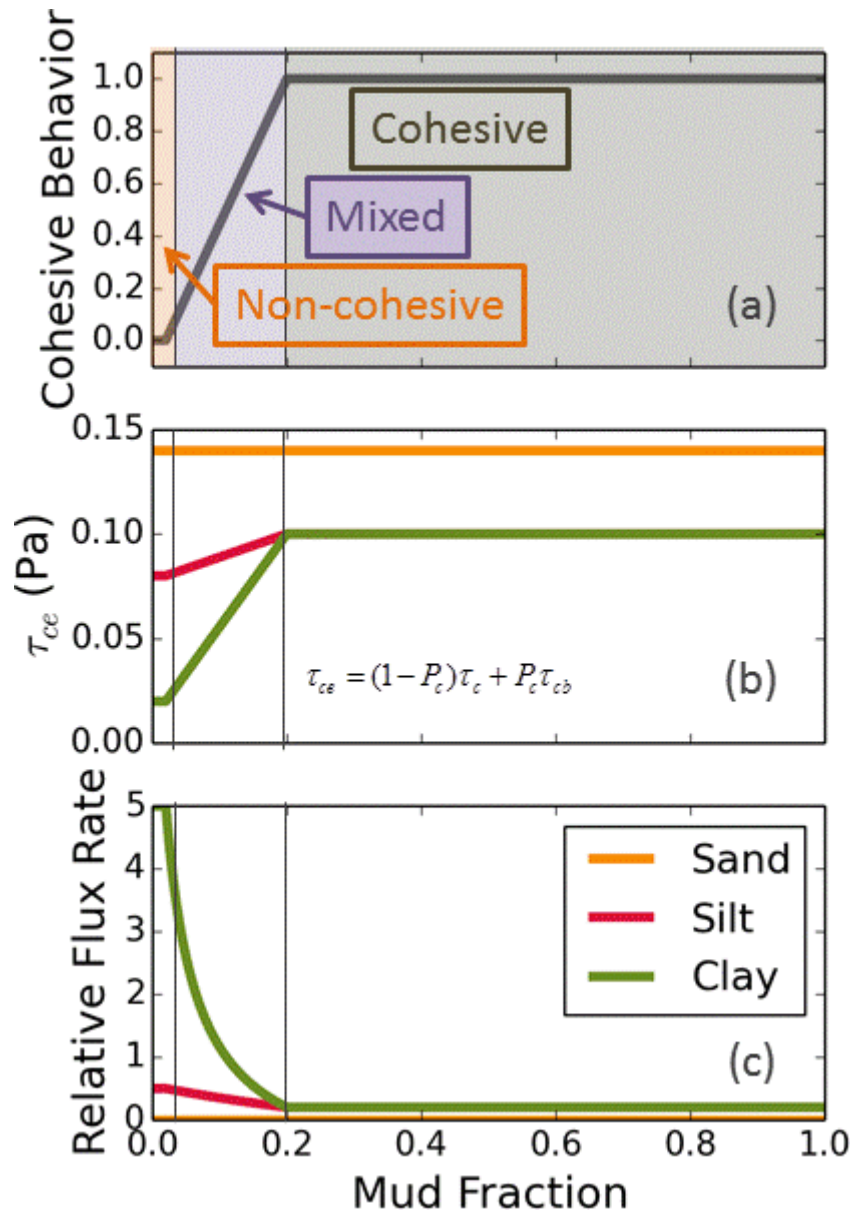


Figure 2. Summary of mixed-bed behavior with increasing of mud fraction f_c (the combined mass fraction of material in cohesive classes). (a) Cohesive behavior parameter P_c as a function of f_c . (b) Effective critical shear stress τ_{ce} for size classes where bulk critical shear stress of the bed $\tau_{cb} = 0.1$ Pa. (c) Relative flux (normalized excess shear stress) from the bed when bed stresses are $\sim \tau_b = 0.12$ Pa (greater than τ_c for clay and silt primary particles, but less than τ_c for sand)

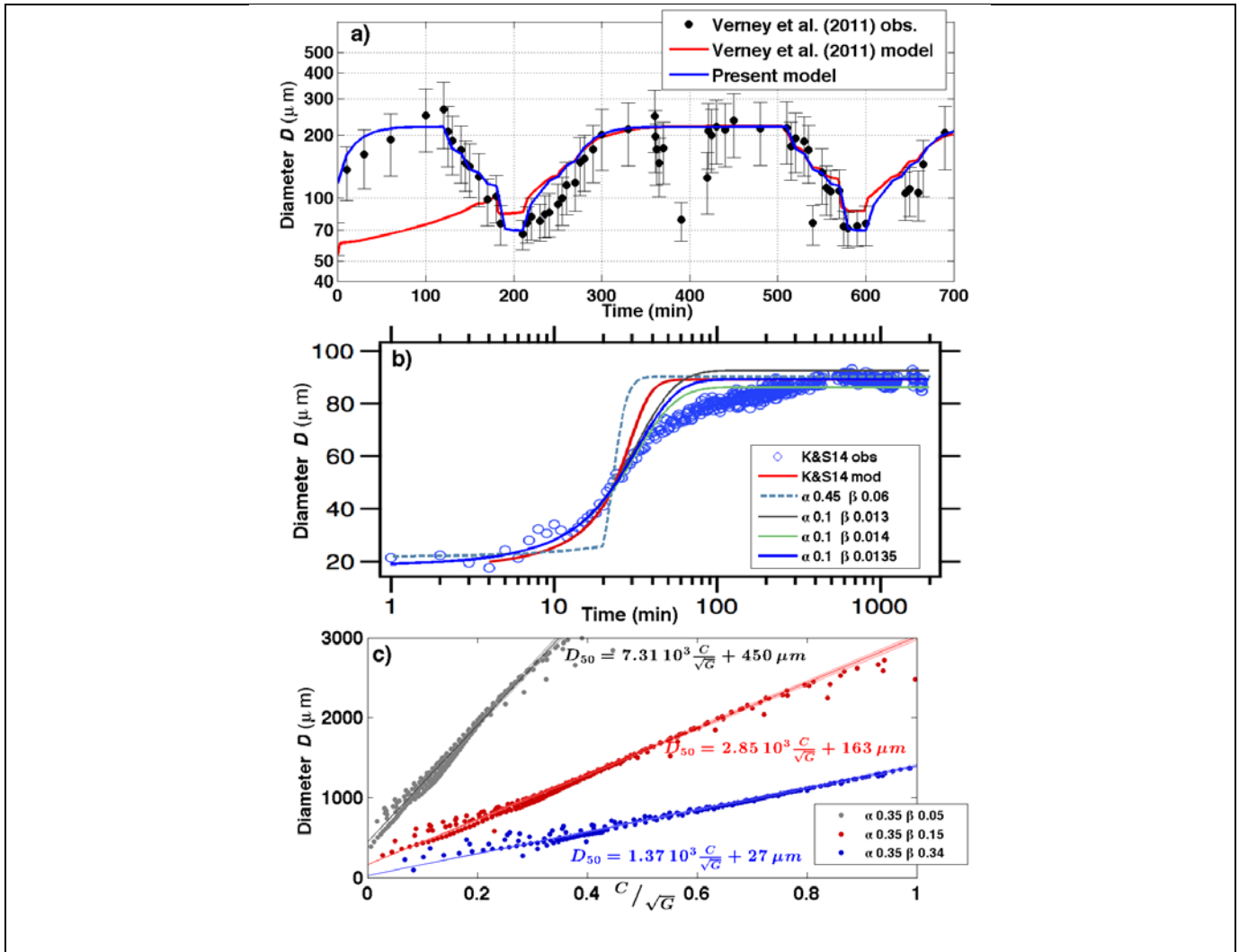


Figure 3. Comparison of ROMS implementation of FLOCMOD with laboratory and theoretical results. (a) Laboratory response of floc size to simulated fluctuations in shear rate G showing observed area-weighted floc diameter D (black dots with error bars), model results presented in Verney et al., (2011; red line), and ROMS FLOCMOD simulation (blue line). (b) Laboratory response of floc size to rapid increase in shear rate from $G=0$ to $G=15 \text{ s}^{-1}$ showing sizes measured by Keyvani and Strom (2014; K&S14; blue circles), K&S14 model results (red line), and ROMS FLOCMOD results for various combinations of aggregation and breakup parameters (dashed and colored lines). (c) Equilibrium diameters produced by steady ROMS FLOCMOD simulations with a range of concentrations, shear rates, and aggregation and breakup parameters (dots). These fall along lines with slopes determined by the ratio of aggregation and breakup parameters, according to theory (Winterwerp, 1998).

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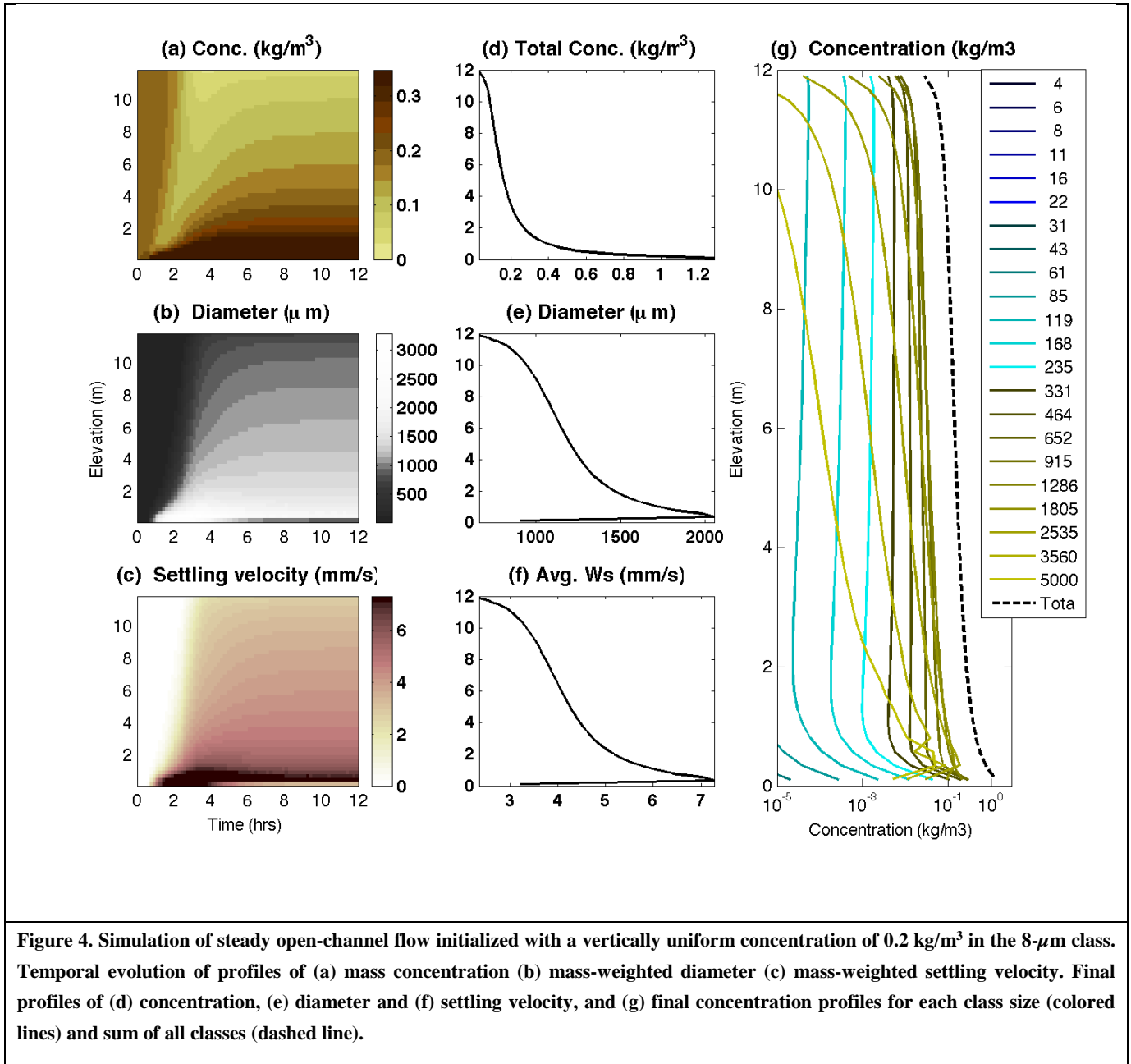


Figure 4. Simulation of steady open-channel flow initialized with a vertically uniform concentration of 0.2 kg/m^3 in the $8\text{-}\mu\text{m}$ class. Temporal evolution of profiles of (a) mass concentration (b) mass-weighted diameter (c) mass-weighted settling velocity. Final profiles of (d) concentration, (e) diameter and (f) settling velocity, and (g) final concentration profiles for each class size (colored lines) and sum of all classes (dashed line).

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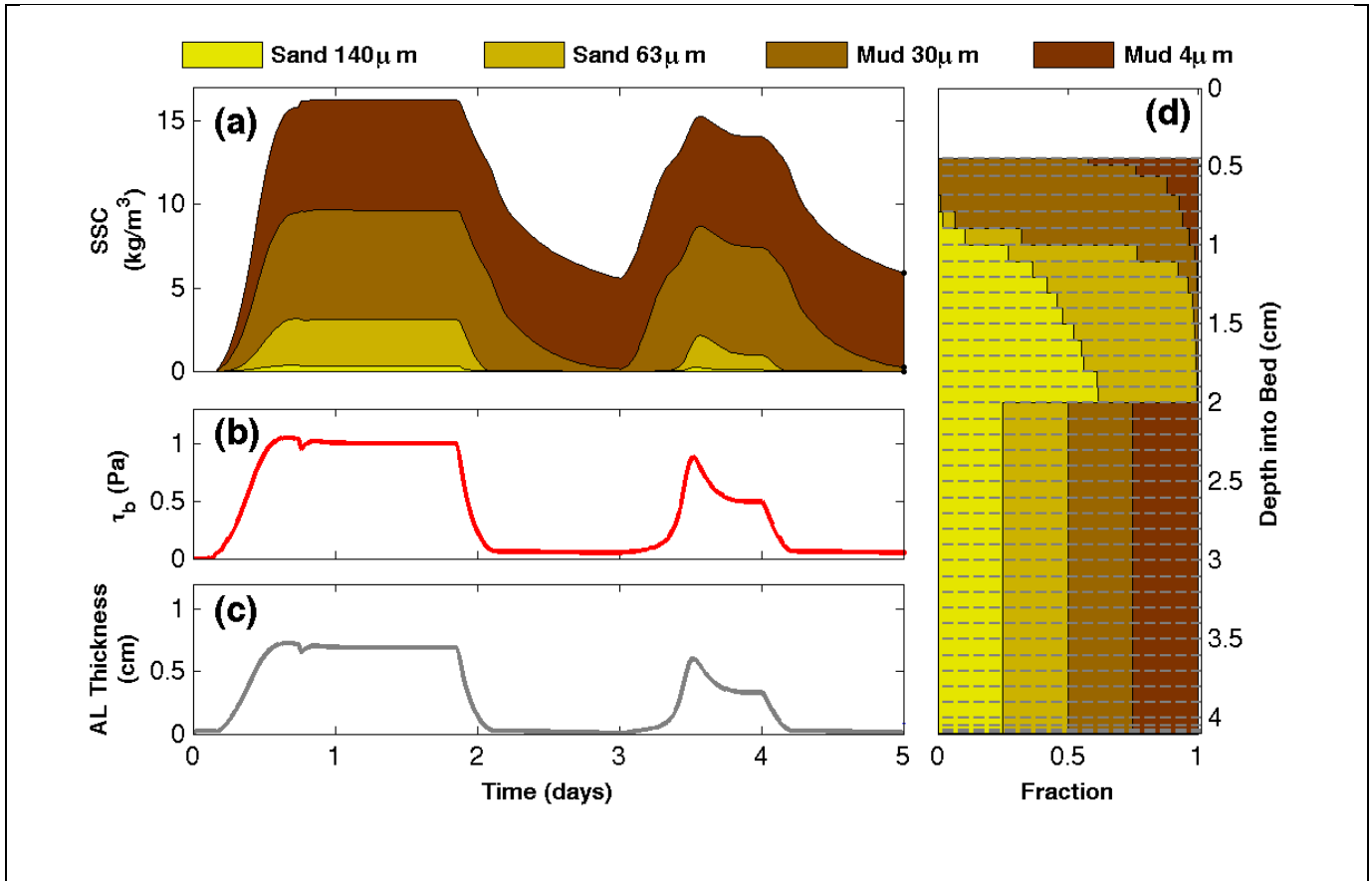


Figure 5. Summary of the double resuspension experiment with non-cohesive sediment over 5 days. The model setup included 41 bed layers, a minimum new layer thickness of 1 mm, and four non-cohesive classes. The top horizontal panel (a) shows the time evolution of the mass of sediment in suspension, colored by size class. The middle horizontal panel (b) is the time series of bottom stress, and the bottom horizontal panel (c) shows the corresponding time series of active-layer thickness. The right panel (d) depicts the final stratigraphy relative to the initial bed level at zero and shows the fraction of each sediment class in each bed layer.

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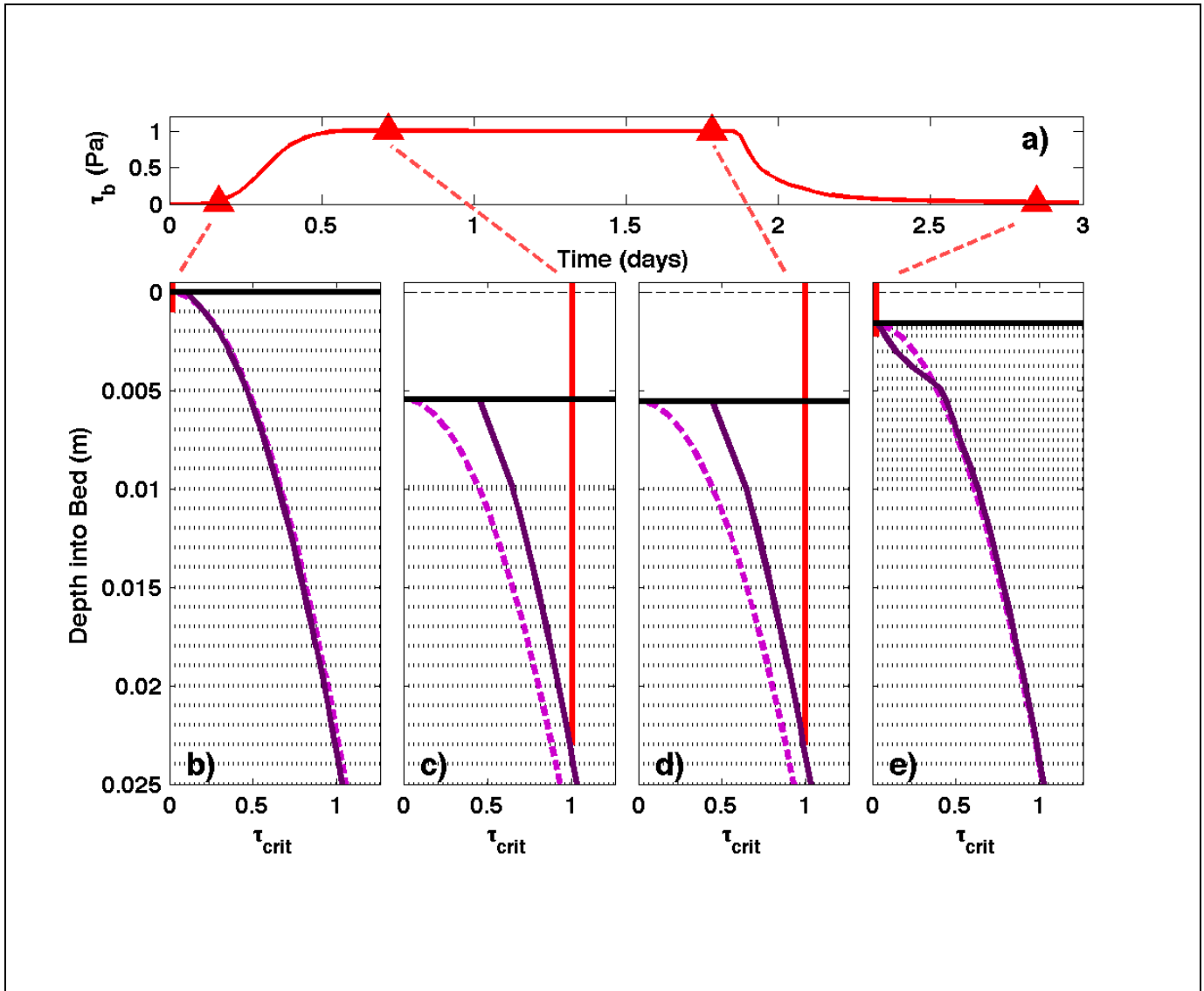
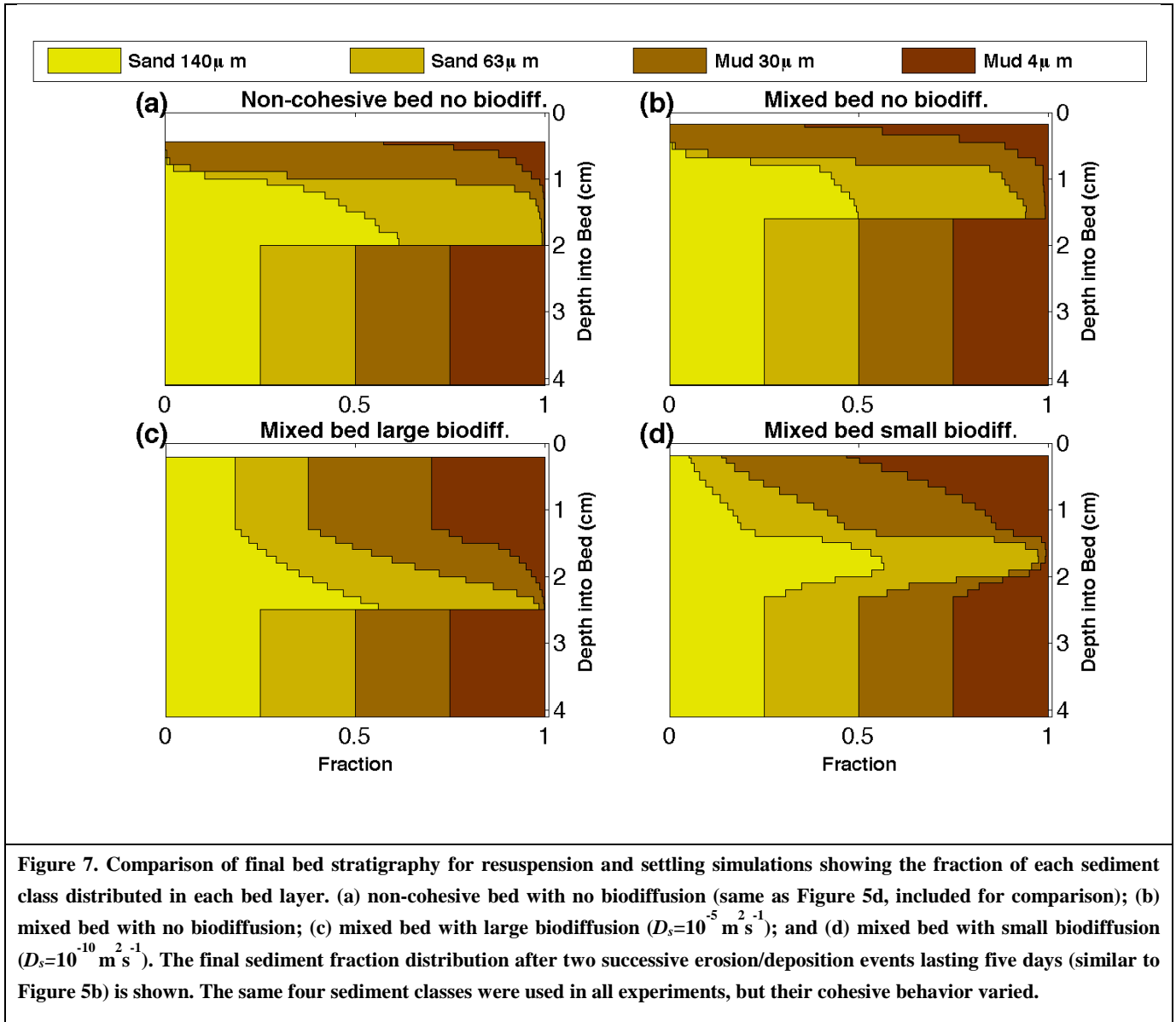


Figure 6. Time series of bottom stress (a) and profiles of critical shear stress for erosion during four distinct conditions: (b) initial bed condition; (c) eroded bed (after 0.7 days with $\tau_b = 1.0$ Pa); (d) unchanged bed level but modified bulk critical stress profile after 1.2 additional days with $\tau_b = 1.0$ Pa); and (e) deposition after a day of low stress with $\tau_b = 0.1$ Pa). In the lower panels, the solid red line is the magnitude of the bottom stress (τ_b), the dashed magenta line is the equilibrium profile of bulk critical stress for erosion $\tau_{c,b}(z)$, and the solid purple line is the instantaneous profile of bulk critical stress for erosion. The solid black line is the instantaneous position of the top of the bed at each time, with the initial bed elevation starting at zero.

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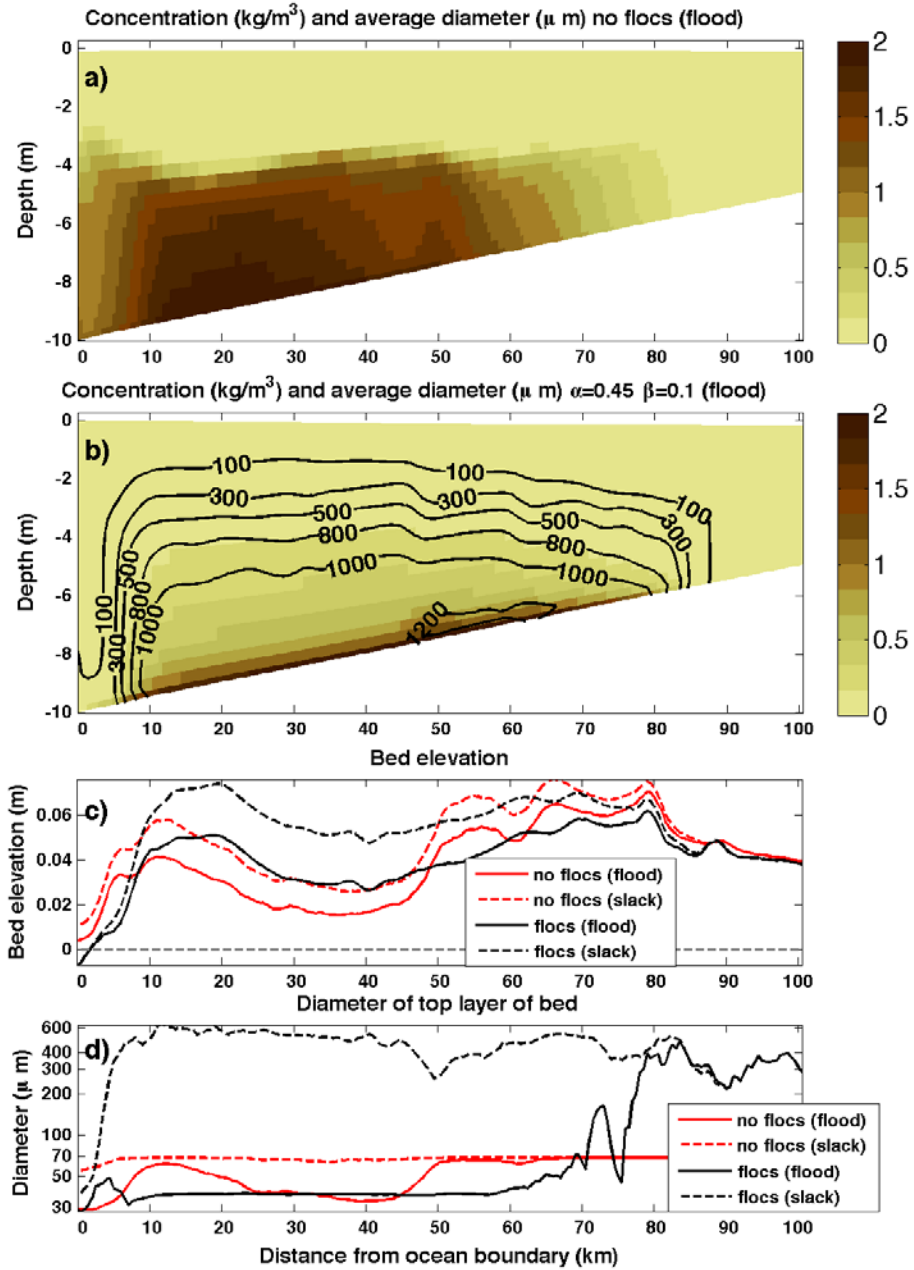


Figure 8. Comparison of estuarine turbidity maxima simulations with and without floc dynamics. The model was initialized with a uniform suspended-sediment concentration of 0.1 kg/m^3 in the $37\text{-}\mu\text{m}$ class.



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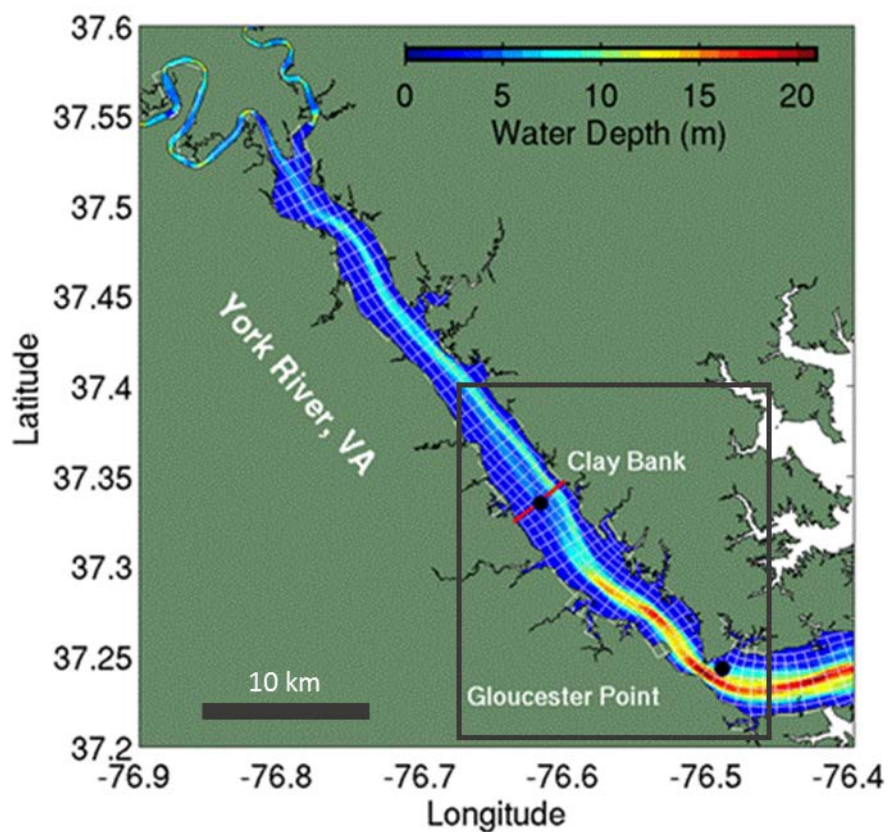


Figure 9. York River bathymetry (color scale), and model grid (white lines show every fifth grid line in the along- and across-channel directions). The region outlined in grey is expanded in Figure 10.

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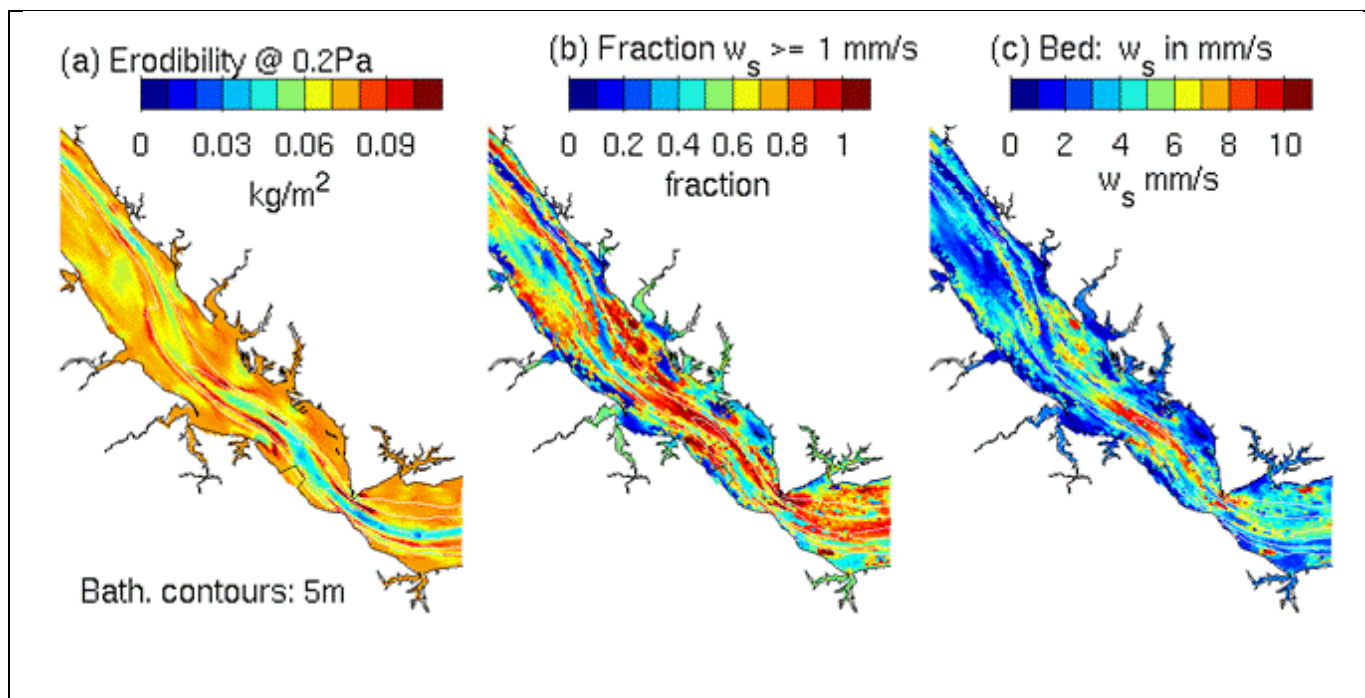


Figure 10. Model estimates of seabed properties after two months of tidal forcing and constant, average freshwater discharge. (a) Erodibility of the seabed, calculated as the thickness of the layer having a critical shear stress exceeded by 0.2 Pa. (b) Fraction of the surficial sediment in the “faster settling” size class. (c) Average settling velocity of surficial sediment.

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